



Hawea Dam Break Update
Contact Energy Limited
30 September 2011

Hawea Lake Control Dam

Dam Break Analysis Update Report

AECOM

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Executive Summary

Contact Energy has engaged AECOM to undertake a hydraulic modelling study investigating potential consequences that could result from a failure of the Lake Hawea Control Dam. This report and accompanying flood maps document the results. A previous dam break study was undertaken in 1990, but in the last twenty years there has been significant development around Wanaka town requiring updated flood maps.

Potential Failure Scenarios and Breach Parameters

Two possible failure scenarios have been modelled; an earthquake induced piping failure under normal operating and inflow conditions (termed the 'sunny day event') and a piping failure induced by high lake levels following a Probable Maximum Flood (PMF) event in the catchment (termed the 'rainy day event').

A sunny day failure scenario was modelled in the 1990 study and has been repeated in this study using the same breach parameters. These being a single failure of the east dam only with a base breach width of 30m and breach invert of RL 326.00m. The starting water level was RL 346.00m, which is the maximum normal operating level.

A rainy day event was considered because of the New Zealand Society on Large Dams (NZSOLD) guideline requirement to model both fair weather and design flood scenarios. For the Hawea Dam the design flood is the PMF. The most recent PMF study for the Clutha catchment (Opus 2000) suggests that following a PMF event the level of Lake Hawea would be higher than the dam core for more than ten days. A piping failure was modelled with the same breach dimensions as the sunny day event, but with a starting lake level of RL 350.24m.

An analysis of a failure of the Gladstone Gap stopbank was outside the scope of this work and is not considered in this study. A Gladstone Gap failure event has previously been modelled by Opus (2007).

Breach Outflows

The peak breach outflow from Lake Hawea was estimated to be 14,500m³/s and 21,300m³/s for the sunny and rainy day failure events respectively. Because of the very large volume of water stored in Lake Hawea the breach discharge would continue for several weeks with the outflow gradually reducing as the lake level dropped until the lake was fully drawn down to a water level of RL 326.00m.

Flood Wave Hydraulic Modelling

The dam breach and downstream flood wave have been modelled simultaneously using HEC-RAS (version 4.1.0). Because of the deeply entrenched nature of the Hawea and Clutha Rivers a one-dimensional modelling approach was appropriate.

The model was developed using cross sections of the Clutha River supplied by Contact. These sections are a mix of ground survey for the river channel and LIDAR survey for the floodplain. The Hawea River was represented in the model using cross sections extracted from LIDAR data supplied by Contact.

Lakes Hawea, Wanaka and Dunstan were all represented using level-pool storage nodes. The Hawea and Clyde Dams were included in the model with stage-discharge curves based on Contact's Clutha flood rules.

Sensitivity Analyses

A wide range of sensitivity analyses were conducted to test the effects on the initial breach discharge and the downstream flood wave propagation. The peak discharge was found to be insensitive to changes in breach formation time, with formation times ranging between 30mins and 3hrs giving a change of less than 2%. The model is not sensitive to breach formation time because of the large storage volume and the time required to draw down the lake.

Both the sunny and rainy day models were run with a starting water level of RL 277.20m in Lake Wanaka. This is the median lake level for the period 1930-2007 (Opus 2009) and results in minimal inundation through Wanaka town. For a rainy day event it is likely that Lake Wanaka would have a higher initial level so a model was also run with a 100yr ARI starting level of RL 280.87m. This gives a peak lake level of RL 283.45m and significantly increases the inundation and damage through Wanaka town. Peak flood levels also increase in Albert Town.

The model did not include any allowance for sediment or debris transport, instead a sensitivity analysis was run using higher Manning's n values. Using a coefficient of 0.060 for both the floodplain and the channel decreases peak discharge by approximately 4% and increases peak water levels by an average 4% (with a range -2.2% to

+10%). The entrenched river channels mean this increased water level does not correspond to an increase in flood extent.

The model uses the Clyde Dam as the downstream boundary with a discharge rule based on Contact's Clutha flood rules. With these operational rules the dam would be overtopped in both a sunny and rainy day event. A sensitivity analysis considered an alternative scenario where the spillway gates are opened fully when water level in Lake Dunstan exceeds RL 195.10m. This gives negligible reductions in peak water level and discharge for both sunny and rainy day events.

Flood Inundation Extent

Although the Hawea River is deeply incised in its upper reaches, the depth of flood water following a breach means there would be some inundation onto the Hawea Flats downstream of the Hawea oxidation ponds. The bridge at Camp Hill Road would likely be destroyed by the initial flood wave, 40mins after maximum breach formation. With the constrained channel and the high peak discharge there would likely be severe bed and bank erosion along the Hawea River.

Albert Town is located at the confluence of the Hawea and Clutha Rivers and would experience widespread inundation and damage following a failure of Hawea Dam. The low lying area around the confluence would act as a flood storage area and there would be negative flow up both the Clutha and Cardrona Rivers. The flood peak would arrive at Albert Town 2.50 hours after the initiation of a breach at Hawea with the peak water level occurring 1.50 hours later (4.00 hours after breach initiation). The SH6 bridge would likely be destroyed, either by the initial flood wave or by erosion and slope instability on the south bank.

With an initial water level of RL 277.20m in Lake Wanaka and a peak flood level of RL 286.65m (for the sunny day event) in Albert Town, the normal flow direction would be reversed upstream of Albert Town. Water would spill into Lake Wanaka, which would experience a water level rise of 2.02m for the sunny day event and 2.98m for the rainy day event. Lake Wanaka has a surface area of approximately 193km² and this large storage capacity means the lake would act as a flood detention basin following a breach of Hawea Dam. The peak inflow to the lake is 4,076m³/s during a sunny day event and 6,500m³/s during a rainy day event.

The large storage volume means the peak lake level would not occur for 3 days after a Hawea failure. When flood levels in the Clutha begin the drop, Lake Wanaka would then discharge the flood water back into the Clutha River. This increases the total duration of the flood event downstream of Albert Town, but significantly attenuates the peak discharge.

The sensitivity analyses show that if Lake Wanaka had a higher initial water level (close to the 100 year ARI of RL 280.87m) there would be extensive inundation and damage through Wanaka town. The lake level rises by 2.58m to a peak of RL 283.45m. Approximately 90 buildings, including several hotels, numerous cafes and a supermarket would all be inundated.

Downstream of Albert Town the peak flood discharge would be attenuated by both the ponding through Albert Town and the negative flow upstream to Lake Wanaka. The Clutha River is deeply incised and the flood wave would be contained within the channel. State Highway 8A crosses the Clutha River at Luggate, approximately 30km downstream of the Hawea Dam and the bridge would likely be destroyed in both a sunny and rainy day event.

The peak water levels in Lake Dunstan are controlled by discharge from the Clyde Dam. During a sunny day event and assuming Contact's flood rules were followed and the initial lake level was RL 194.00m, Lake Dunstan would rise by 3.74m to a peak of RL 197.74m. The corresponding figures for a rainy day event are 5.59m and RL 199.59m. The crest level of Clyde Dam is RL 196.75m, but the dam has been designed to withstand overtopping so a cascade failure is unlikely.

The population at risk (PAR) following a failure of Hawea Dam is estimated to be 2,040 for the sunny day event and 2,280 for the rainy day event. If a rainy day event were to occur in coincidence with a high water level in Lake Wanaka, the PAR could increase to 2,620.

1.0 Introduction

1.1 Overview

The primary objective of this study was to identify the potential impacts and consequences of a flood wave in the Hawea and Clutha Rivers resulting from a hypothetical failure of Lake Hawea Control Dam. This includes analysis of both sunny day and rainy day failure events and production of detailed flood inundation maps.

A previous dam break study was undertaken in 1990, but in the last twenty years there has been significant development in Wanaka and Albert Town requiring updated and more detailed flood maps.

1.2 Description of Dam and River Reach

The Hawea Lake Control Dam is a zoned earth fill dam blocking the outlet of Lake Hawea. The dam has a height of 25m and is constructed in two parts, east and west separated by a central rock island. The east dam blocks the original outlet channel and contains the outlet structure, which comprises four low level sluice gates each with a maximum discharge capacity of 141m³/s (Worley, 1998). The west dam is smaller and blocks a diversion channel that was used during construction of the dam.

Lake Hawea has an operating range of 8m with a normal maximum operating level of RL 346.00m. At this elevation the lake has a storage volume of 2,531Mm³ and a surface area of approximately 149km². Lake Hawea is an old glacial lake and the Hawea Dam is a short extension of the long natural barrier formed by post glacial action (Opus 2009).

In addition to the control dam a smaller stopbank has been constructed 3km to the east of the control dam. The stopbank is known as the Gladstone Gap and has a crest elevation of RL 350.49m. The stopbank could be deliberately breached during a large flood to increase discharge from the lake and relieve pressure on the control dam. A stopbank breach would inundate large sections of the Hawea flats and would only reduce the lake level to RL 349.00m (the minimum stopbank level). Table 1 gives the key parameters for the Hawea and Clyde Dams.

Table 1 Hawea and Clyde Dam Parameters

	Hawea Dam	Clyde Dam
Dam type	Earth fill	Concrete gravity
Dam height (m)	25	100
Dam crest level (m RL)	351.40	196.75
Dam core level (m RL)	349.65	-
Minimum normal operating lake level (m RL)	338.00	193.50
Maximum normal operating lake level (m RL)	346.00	194.50
Storage volume at maximum operating level (Mm ³)	2,531	387
Design flood level (m RL)	349.05	195.10

Downstream of the control dam the Hawea River is deeply entrenched for most of its 14km length to the confluence with the Clutha River at Albert Town. The geology of the Hawea River is old fluvio-glacial deposits that could be susceptible to erosion during high large flood events.

The Clutha River drains from Lake Wanaka, 5km upstream of the Hawea confluence. Lake Wanaka has a surface area of 193km² and live storage volume of 309Mm³ at RL 277.0m. At RL 272.0m the lake outlet is only 3m higher than the channel bed at Albert Town.

Downstream of Albert Town the Clutha continues to be deeply entrenched as far as the confluence with the Lindis River. Downstream of the confluence the river valley widens and the river has a wide braided bed. As the river approaches Lake Dunstan there are several small islands in the river formed by sediment deposition at the head of the lake.

The Clutha River flows into the Clutha arm of Lake Dunstan, which is impounded behind the Clyde Dam. The Clyde Dam is a 100m high concrete gravity dam and is located 94km downstream of the Hawea dam. The lake has a 1.0m operating range with a maximum normal operating level of RL 194.50m and a total storage volume of 387Mm³.

1.3 Previous Studies

The previous Hawea dam break study was undertaken by Works Consultancy Services in 1990. The objective of the study was to identify the impact of a failure of Hawea dam on Clyde Dam, which was under construction at the time. The study considered a sunny day failure resulting from an earthquake induced piping failure; a rainy day failure was excluded from the analysis on the basis that the large storage volume in the lake meant it was very unlikely the dam would be overtopped.

The 1990 study used the USBR DAMBRK software program to model the breach outflow hydrograph with the downstream flood wave modelled using a water balance approach.

The results of the 1990 study are summarised and compared to the results of this study in Section 6.7.

2.0 Consideration of Potential Failure Scenarios

When assessing the potential consequences of a dam failure it is standard practice, both within New Zealand and worldwide to consider two hypothetical scenarios, which are often termed "sunny day" and "rainy day" failures. The potential failure scenarios considered in this section relate specifically to embankment dams only.

Dam break modelling is standard practice for all dams and is undertaken for emergency planning purposes. Dam break modelling is an analysis of a hypothetical event and is not indicative of any actual concerns at Hawea Dam. In New Zealand it is an NZSOLD requirement to determine potential inundation areas for emergency planning purposes.

A sunny day event represents a dam breach under normal flow and operating conditions, which may occur as a result of internal piping through the embankment, embankment instability or leakage from internal structures (e.g. conduits through the structure). These failure mechanisms could result from structural damage following seismic events. Overtopping failures are typically not considered during a sunny day analysis but could occur as a result of prolonged blockage of the outlet structures.

A rainy day failure represents a dam breach resulting from a flood event, which would most likely occur due to overtopping of the structure either as a result of spillway blockage or lack of capacity. Flood events in excess of the design event (for example, a probable maximum flood) are considered and failure is modelled at the maximum reservoir level.

The following sections discuss hypothetical sunny and rainy day failure mechanisms for the Hawea Dam.

2.1 Sunny Day Failure

Several potential sunny day failure scenarios were outlined in the 1990 study. These have been reviewed as part of this study and are considered to remain valid. They are summarised in the following paragraphs.

- A large earthquake reactivating the Mt Maude slide, with the subsequent slide movement squeezing the west part of the dam, cracking the dam fill and initiating a piping failure.
- A large nearby earthquake imposing a shearing action on the east part of the dam, cracking the dam fill and initiating a piping failure.
- An earthquake causing a rupture of the irrigation pipe running through the east part of the dam with the internal seepage causing a piping failure. This scenario would require a high lake level to develop a serious failure.

The 1990 study concluded that a piping failure through the east dam as a result of an earthquake would produce the largest breach size and potentially the most severe consequences. This hypothesis is still valid and has been used in this study as a sunny day failure mechanism.

2.2 Rainy Day Failure

The 1990 study concluded that there is sufficient discharge capacity through the dam and storage capacity in the lake that an overtopping failure had a lesser probability than a piping failure by an order of magnitude. On this basis the 1990 study did not consider a rainy day failure event. However the NZSOLD guideline now requires that a rainy day event is modelled.

The Clutha Hydro Dams PMF Update (Opus 2000) modelled a Probable Maximum Flood (PMF) event in the Hawea catchment using three different starting lake levels. This confirmed that even with an initial lake level of RL 346.00m (the normal maximum operating level) there is sufficient storage volume in the lake that neither the dam nor the Gladstone Gap stopbank would be overtopped. For the worst case scenario the lake level peaks at RL 350.37m, which is 1.03m below the dam crest but 0.72m above the core level. With the modelled lake level remaining higher than the core level for at least ten days there is the possibility that a seepage induced failure could occur under PMF conditions.

The core level has a higher elevation than the rock island separating the east and west dams, so seepage induced failure could occur on either side of the dam. A failure of both the east and west dams was not considered, on the basis that both would not fail simultaneously and once one side of the dam failed the downstream flood wave and increased tail water level would equalise the hydrostatic pressure on the remaining dam, reducing the possibility of complete failure. As the east dam is the larger of the two and would produce the largest breach size (and hence the largest peak discharge), only a failure of the east dam was modelled.

A rainy day failure resulting from a PMF in the wider Clutha catchment was not considered because peak water levels in Lake Hawea would not exceed core level.

3.0 Breach Outflow Estimates

The dam breach and downstream flood wave have been modelled simultaneously using HEC-RAS (version 4.1.0) hydraulic modelling software. HEC-RAS is developed by the US Army Corps of Engineers (USACE) and is a one-dimensional hydraulic modelling package. The software has the capability to dynamically model both the dam breach and the resulting downstream flood wave.

3.1 Breach Parameters

The breach parameters have been considered with reference to the 1990 study and published literature. Table 2 shows the breach parameters estimated using equations from eight published studies referenced in USBR (1998), all considering embankment dam failures. These parameters have been estimated for a failure of the eastern dam following a rainy day event.

Both the sunny and rainy day events have been modelled as piping failures with the same maximum breach dimensions.

Table 2 Rainy Day Breach Parameters (estimated from USBR Literature)

Parameter	Symbol	Units	MacDonald and Langridge Monopoles (1984)	USBR (1901)	Von Thun and Cichle (1993)	French (1992a)	National Weather Service (1992)	National Weather Service Simplified Method	Wilder and O'Connell (1997)	Webby (1996)
Average breach width	B	(m)	-	73.11	115.83	275.44	166.68	76.11	-	-
Upper breach width	B_{top}	(m)	-	97.97	155.21	374.80	225.35	-	-	-
Lower breach width	B_{bottom}	(m)	-	48.25	76.44	176.28	115.01	-	-	-
Breach depth	h_o	(m)	25.37	25.37	25.37	25.37	25.37	25.37	-	-
Breach formation time	t_f	(hrs)	4.1	0.8	0.7	-	7.4	8.1	-	-
Volume removed	V_{er}	(m^3)	3,103,710	-	-	-	-	-	-	-
Side slope ratio		(1h xv)	2.00	1.00	1.00	0.90	1.00	-	-	-
Peak discharge	Q_p	(m^3/s)	-	-	-	15,566	-	-	18,360	26,885

Of these, the key parameter in the case of Hawea Dam is the breach formation time, which has a range from 42 minutes to 8 hours. Other parameters, such as maximum breach width and breach depth are constrained by the

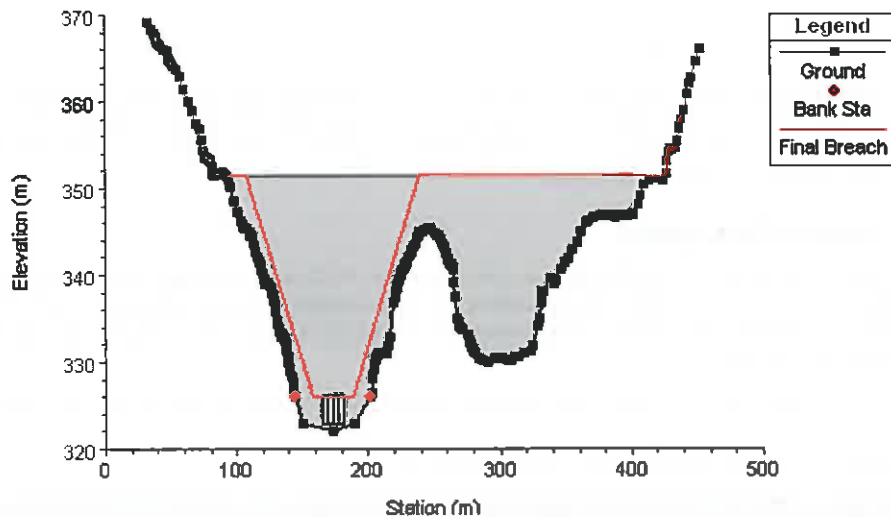
geometry of the east dam, as shown on Figure 1. The minimum breach elevation of RL 326.00m is just above the top of the sluices. A bottom breach width of 30m was used.

The parameters used in the modelling are shown in Table 3. The base case breach models use a breach formation time of 1.0 hours, but a sensitivity analysis was also done using a range between 30mins and 3 hours (see Section 5.1.1).

Table 3 Modelled Breach Parameters

Parameter		Sunny Day	Rainy Day
Initial Lake Level	m RL	346.00	350.24
Peak Inflow to Lake	m ³ /s	50	5,440
Failure Method		Piping	Piping
Time to Maximum Breach Formation	hrs	1.0	1.0
Final Breach Invert	m RL	326.00	326.00
Initial Piping Elevation	m RL	326.00	349.65

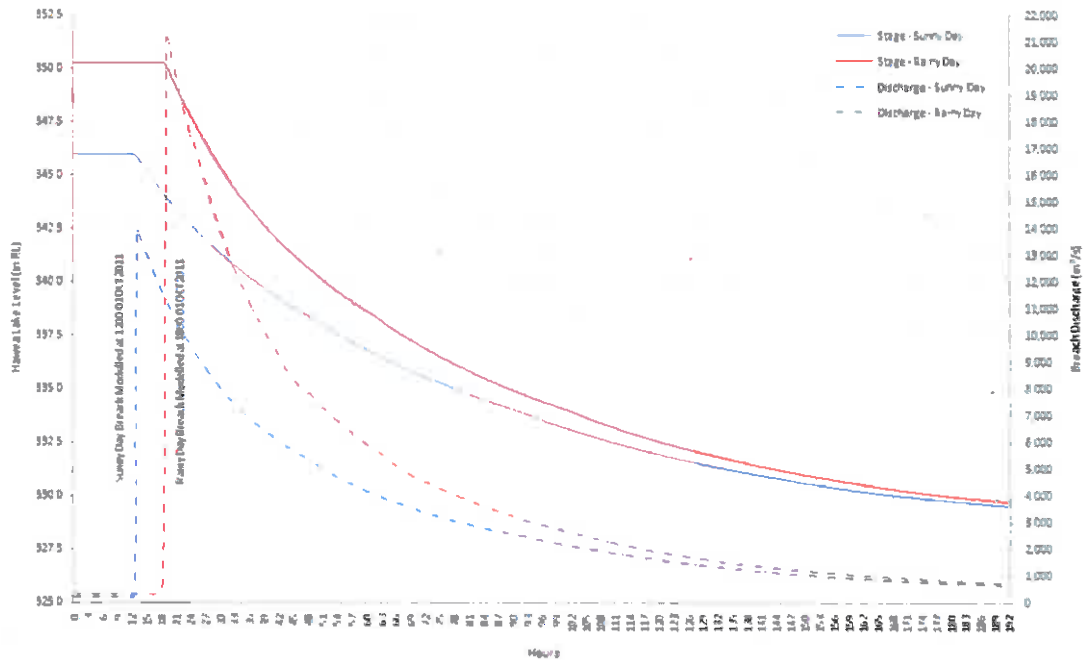
Figure 1 Final Breach Dimensions, Hawea Dam (looking downstream)



3.2 Breach Outflow

The modelled breach hydrographs for both a sunny and rainy day failure are shown in Figure 2. The peak discharge is 14,500m³/s and 21,300m³/s for the sunny and rainy day events respectively.

Figure 2 Modelled Breach Outflow and Stage Hydrographs, Lake Hawea



4.0 Flood Wave Hydraulic Modelling

4.1 Methodology

The hydraulic model has been built to include lakes Hawea, Wanaka and Dunstan with the three main river reaches connecting them (the Hawea River, Clyde River upstream of Albert Town and Clyde River downstream of Albert Town). The model was developed using GIS data and the HEC-GeoRAS extension. The extents of the three lakes, river alignments, cross section locations and extents and structure (Hawea and Clyde Dams and Albert Town bridge) locations were all created using HEC-GeoRAS and exported into HEC-RAS.

A Digital Terrain Model (DTM) of the Hawea and Clutha rivers was built using LiDAR data supplied by Contact. This DTM was used to extract cross sections of the Hawea River and to map the flood extents.

The model was initially run without a dam breach for both the sunny and rainy day (PMF) events. These base case model runs were used to verify the modelling parameters, particularly channel and floodplain friction coefficients and boundary conditions (both external boundaries and internal conditions at structures).

Because of the inherent uncertainties involved in dam break modelling, a thorough sensitivity analysis was conducted for both the sunny and rainy day breach models. The model parameters that were considered during the sensitivity analysis included breach formation time, breach progression type, hydraulic roughness (Manning's n) and initial lake levels. A detailed discussion of the sensitivity analysis is provided in Section 5.0.

Flood inundation extents for the sunny and rainy day events were generated in Arc-GIS (version 9.3) using the GIS based Hec-GeoRAS post-processing tools.

4.1.1 Model Extents

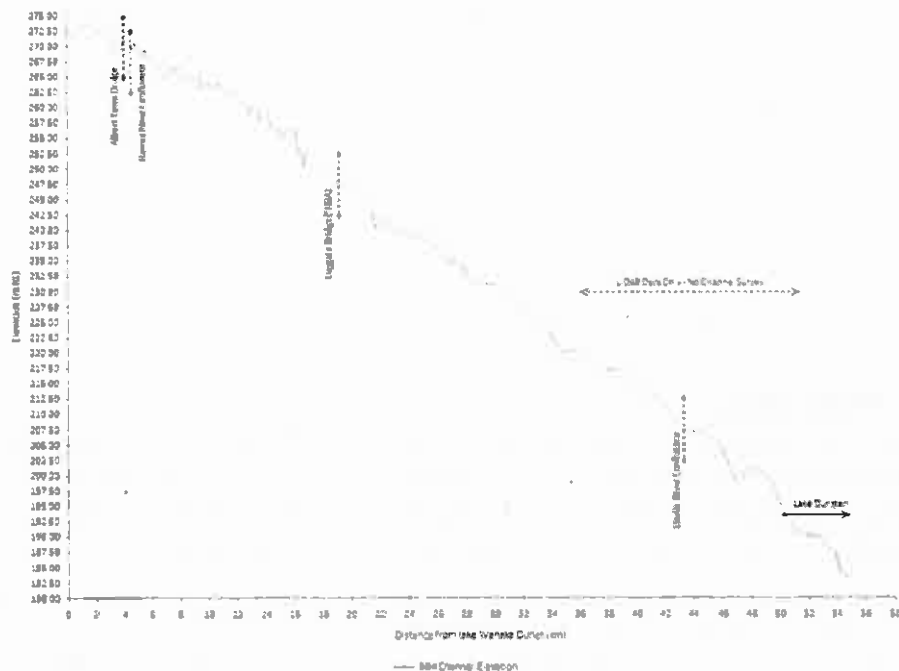
The bounding extents of the hydraulic model are Lakes Hawea and Wanaka at the upstream extents and the Clyde Dam at the downstream extent. The extent of the model (including cross section locations) is shown in the Figure in Appendix C.

4.1.2 Topographic Data

The model was developed using a mix of surveyed cross sections and LiDAR survey. Cross sections for the Clutha River between Lake Wanaka and Lake Dunstan were supplied by Contact and comprised surveyed channel sections with floodplain extents represented using LiDAR survey. The channel and LiDAR survey was supplied in DHI MIKE11 export format along with a cross section coordinate file so the cross sections could be geo-referenced in HEC-RAS. In total 114 cross sections were used to represent the Clutha River. A profile of the Clutha River is shown in Figure 3

No channel survey was available for the Hawea River. The Hawea River cross sections use LiDAR survey only and were created in GIS using the Hec-GeoRAS tools. The LiDAR survey was supplied by Contact and was flown in May 2009. Because no channel survey was available the Hawea River sections do not include any elevation data below the water line on the day the LiDAR was flown. Given the large discharge and water depths resulting from a dam breach the lack of channel survey is not considered a significant issue with regard to the modelled flood extents.

Figure 3 Upper Clutha Long Section



4.1.3 Structures

In addition to the two dams (Hawea and Clyde) there are also three road bridges within the model extents. These are the Camp Hill Road bridge crossing the Hawea River, the SH6 bridge at Albert Town crossing the Clutha River and the Church Road bridge at Luggate crossing the Clutha. All three of these bridges were visited during the site visit and are summarised in Table 4.

The Camp Hill Road bridge is a clear span, single lane structure with a deck elevation of RL 317.65m (estimated from the LiDAR survey). Clearance from the bridge soffit to the channel bed is estimated to be approximately 7m, based on an approximate bed level of RL 310.40m (taken from NIWA gauging data at the bridge). This bridge was not included in the modelling as it is considered highly likely the bridge would be destroyed by the initial flood wave.

The SH6 road bridge at Albert Town has four bridge piers (570mm diameter), a deck elevation of RL 279.80m and a soffit elevation of RL 278.10m (structure depth 1.70m). The bridge has a clearance of approximately 7.20m to the channel bed, however there is also an extensive floodplain on the left bank of the river and secondary flow could be expected when water levels exceed RL 277.00m. The bridge is located immediately upstream of the Clutha-Hawea confluence and although the bridge would be overtopped during a breach, peak velocities would be low. Therefore the bridge was included in the model using cross section data supplied by Contact.

The Church Road bridge at Luggate is a single lane truss structure. The bridge has two large diameter (estimated 1.0m+) piers, one adjacent to each bank. The bridge deck elevation is estimated to be RL 257.90m with a clearance of approximately 8.0m (both estimated from the LiDAR survey). This bridge is located on a bend of the Clutha River and there is a flow constriction. The bridge was not included in the modelling as it was considered highly likely it would be destroyed by the initial flood wave.

Table 4 Bridges on the Hawea and Clutha Rivers

River	Structure	Distance from Hawea Dam (km)	Deck Elevation (m RL)	Clearance to Channel Invert (m)	Included in Model
Hawea	Camp Hill Road bridge	5	317.60	7.00	No
Clutha	SH6 Albert Town bridge	15	279.80	7.20	Yes
Clutha	Church Road bridge	29	257.90	8.00	No

4.1.4 Hydraulic Roughness

The model was initially set up using roughness (Manning's n) values of 0.030 for the channel and 0.045 for the floodplain. These values were used throughout the model and were modified during the calibration process.

The model does not explicitly account for sediment and debris that would be transported during a dam break flood. Instead a sensitivity analysis was undertaken using a higher 0.060 roughness value (see Section 5.1.4).

4.2 Boundary Conditions

The boundary conditions for the sunny and rainy day models are discussed in the following sections and summarised in Table 5.

Table 5 Initial boundary Conditions

Boundary	Sunny Day Failure		Rainy Day Failure	
	Lake Level (m RL)	Inflow (m ³ /s)	Lake Level (m RL)	Inflow (m ³ /s)
Lake Hawea	346.00	50	350.24	307
Lake Wanaka	277.20	170	277.20	170
Lake Dunstan	194.00	220	194.00	220

4.2.1 Lake Hawea

Lake Hawea forms the upstream boundary of the Hawea reach of hydraulic model and is represented using a storage node upstream of the dam structure. The storage volume was input as a stage-volume curve using volumes obtained from the Electricity Authority NZ website (volumetric storage data sets prepared by Opus, 2010). This stage-storage data covers the lake range RL 338.09m to 347.24m and was extrapolated to RL 351.00m to match the dam crest level. Figure 4 shows the live storage volume and discharge curves for the lake and control structure.

The Hawea control dam was included in the model as an inline structure with a crest elevation of RL 351.40m. The geometry of the sluice gates was included in the representation of the dam, however for the dynamic model runs discharge from the structure was controlled using a rule curve supplied by Contact in their Clutha Flood Rules document. In the model this rule curve overwrites the gate operation rules.

For the sunny day model runs the initial lake level was set at RL 346.00m. This is the normal maximum operating level of the lake and is the same initial condition as used in the 1990 study.

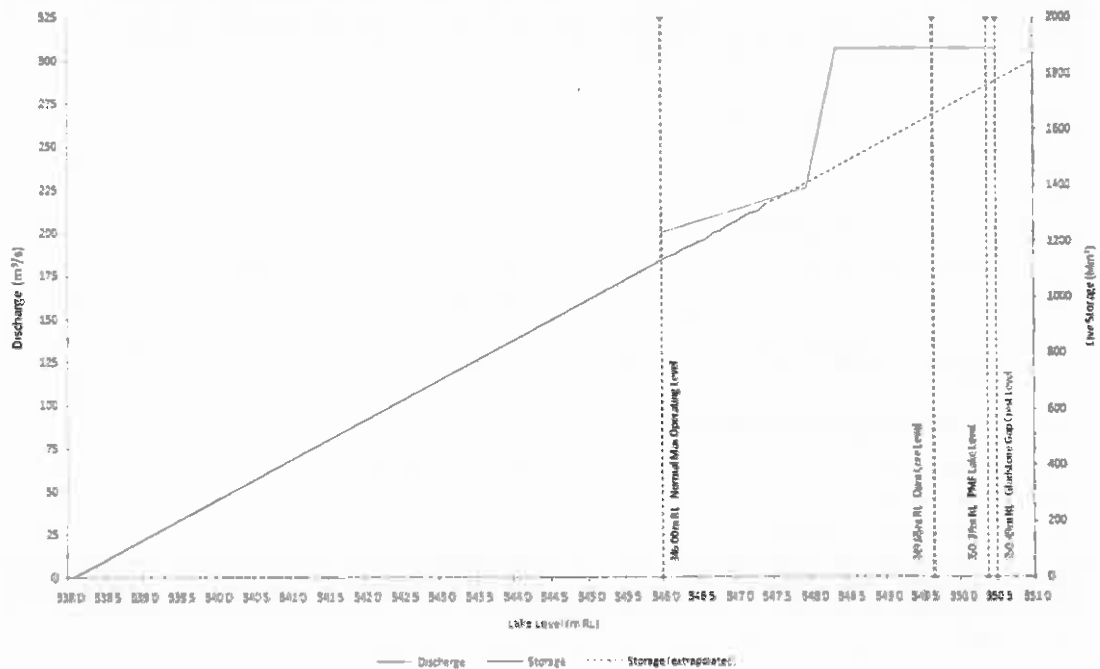
For the rainy day model runs the initial lake level was set at RL 350.24m. This was the peak lake level modelled during the PMF (no breach) model runs and was read into the breach models using a restart file. This starting level is discussed further in Section 4.3.1.

Inflows to Lake Hawea were set at a constant 50m³/s for the sunny day model runs. This is the same value as used in the 1990 study and corresponds to the median lake inflow over the period 1960-2007 (Opus 2009).

For the rainy day model runs the Opus (2000) Case 3, 72 hour PMF hydrograph was used as the inflow condition. Because the breach models were run using a restart file from the PMF (no breach) model run corresponding to the time of peak lake level, only the last 18 hours of the PMF hydrograph were input directly into the breach runs, giving a peak inflow of 307m³/s (which occurs at hour 87 of the full PMF inflow hydrograph).

For the PMF (no breach) model run the full Opus (2000) PMF hydrograph was used as the inflow condition, with a peak inflow of 5,440m³/s.

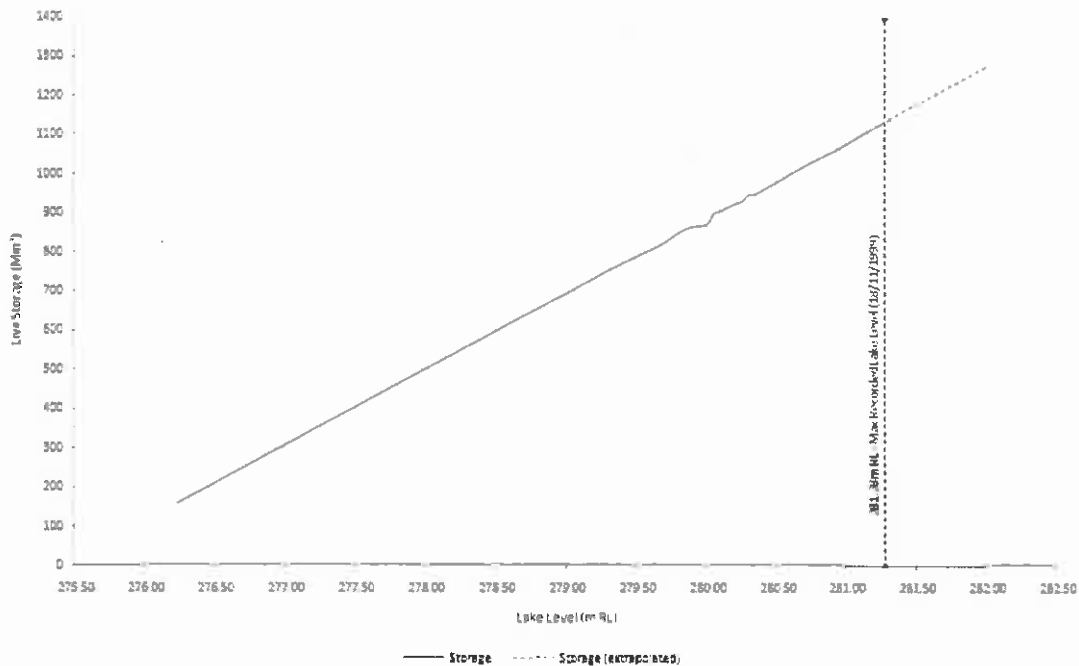
Figure 4 Lake Hawea Stage-Discharge and Live Storage Curves



4.2.2 Lake Wanaka

Lake Wanaka forms the upstream boundary of the Clutha reach of the hydraulic model and is represented using a storage node. The storage volume was input as a stage-volume curve using volumes obtained from the same Electricity Authority NZ (Opus 2010) data set as Lake Hawea. The stage-storage data covers the lake range RL 276.23m to 281.28m and the years 1990-2008. The highest recorded lake level is RL 281.28m, which occurred on 18th Nov 1999; the lake storage has been extrapolated above this level to a maximum of RL 282.00m, as shown in Figure 5.

Figure 5 Lake Wanaka Storage Curve



The initial lake level has been set at RL 277.20m for both the sunny and rainy day breach model runs. This lake level is the same as was used in the 1990 study and corresponds to the median lake level for the period 1930-2007 (Opus 2009). An analysis of the lake record confirms this to be the median level and is considered an appropriate assumption for the initial condition.

Although the rainy day model used a lake level of RL 277.20m as the initial condition, the sensitivity of this assumption was checked using a higher initial lake level of RL 280.87m. This corresponds to the 100yr ARI lake level and is discussed in detail in Section 5.1.3.

Inflows to Lake Wanaka were set at a constant $170\text{m}^3/\text{s}$ for both the sunny and rainy day model runs. Again, this is the same value used in the 1990 study and has been exceeded 30% of the time over the period 1930-2007 (Opus 2009). The sensitivity of inflows to Lake Wanaka was not considered as the direct inflow volume is small when compared to the inflow volume from a failure of Hawea Dam.

4.2.3 Lake Dunstan and Clyde Dam

Lake Dunstan forms the downstream boundary of the Clutha reach of the hydraulic model and is represented using a storage node upstream of an inline structure representing the Clyde Dam. The storage volume of the lake was input using data supplied by Contact covering the range RL 140.00m to 197.00m. The supplied stage-storage curve extends 0.25m above the dam crest and was extrapolated up to a level of RL 200.00m for use in the hydraulic model.

Lake Dunstan has a normal operating range of 1.0m, between RL 193.50m and 194.50m. The initial lake level was set at RL 194.00m for both the sunny and rainy day model runs.

The downstream boundary of the hydraulic model is the Clyde Dam, which has been included as an inline structure with a crest elevation of RL 196.75m. The geometry of the spillway and dewatering gates was included in the representation of the dam, however for the dynamic model runs discharge from the structure was controlled using a rule curve supplied by Contact in their Clutha Flood Rules document. This rule curve overwrites the spillway gate opening rules in the model.

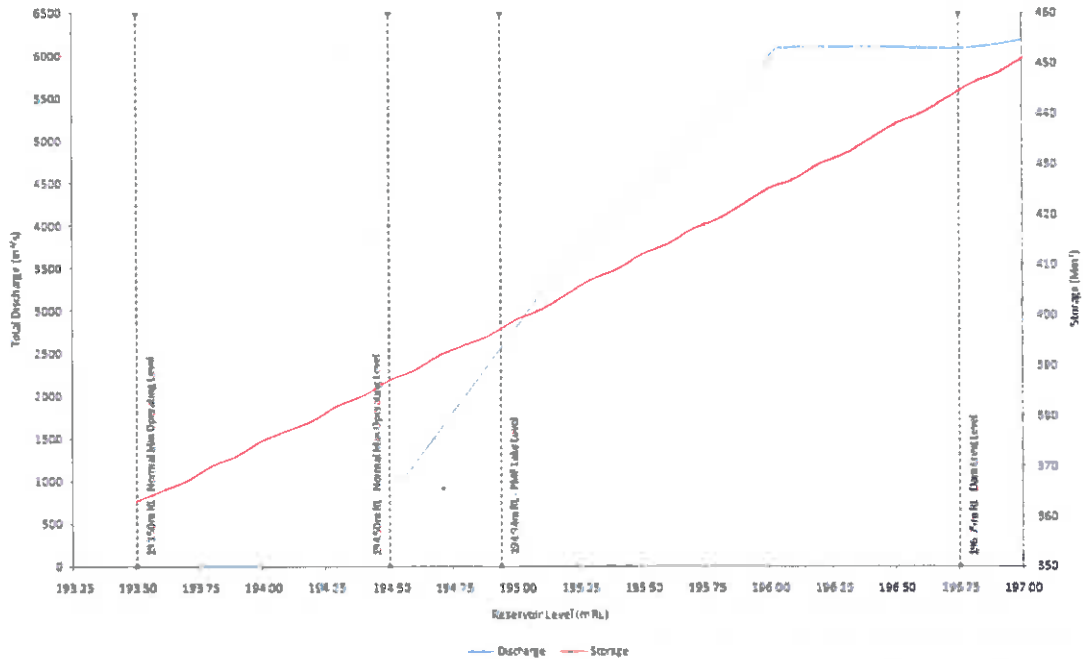
The Lake Dunstan storage curve and Clyde Dam flood discharge rule curve are shown in Figure 6.

The Lake Dunstan storage node has a constant inflow of $220\text{m}^3/\text{s}$ for both the sunny and rainy day events, representing inflow from the Kawarau River and tributaries discharging into the Upper Clutha. With the $190\text{m}^3/\text{s}$ discharge from Lake Wanaka and $200\text{m}^3/\text{s}$ discharge from Lake Hawea (based on the Contact flood discharge

rules for the Hawea Dam), this gives a total steady state inflow of 610m³/s to the lake. This is higher than the mean inflow of 472m³/s (Opus 2007) but is considered a reasonable assumption.

During the PMF (no breach) model run the peak discharge from Hawea Dam is 307m³/s, which increases the total Lake Dunstan inflow to 717m³/s.

Figure 6 Clyde Dam Stage-Discharge and Reservoir Storage Curves



4.3 Model Calibration

The hydraulic model has been calibrated using records from two NIWA stage recorders, located at Camp Hill Bridge on the Hawea River (gauging station number 75287, Mar/68 – Apr/11) and downstream of the Cardrona confluence on the Clutha River (gauging station number 75282, Apr/92 – Apr/11). Additional flood history information has been supplied by Contact (Opus 2000).

The model was calibrated using the three largest events on record at the Cardrona gauge, which occurred on 24th Jan 1994, 13-14th Dec 1995 and 17-18th Nov 1999 and the corresponding water levels in Lakes Wanaka and Hawea. The peak water levels and discharges are shown in Table 6. Lake Hawea discharge information was taken from the ECNZ dataset (Opus, 2010) and is the mean daily discharge.

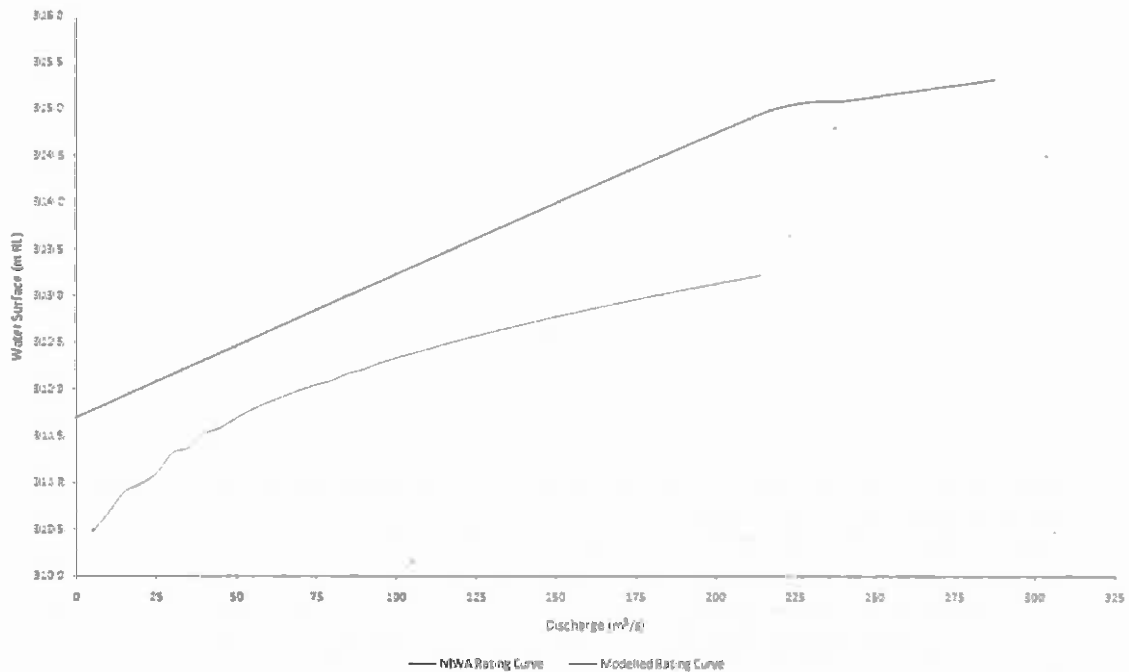
Table 6 Recorded Peak Discharge and Water Levels (NIWA Gauges)

Date	Lake Wanaka		Lake Hawea		Clutha @ Cardrona	
	Peak Level	Discharge	Peak Level	Peak Discharge	Peak Level	
	(m RL)	(m ³ /s)	(m RL)	(m ³ /s)	(m RL)	
24 th Jan 1994	279.72	289	346.07	1,092	274.59	
13 th Dec 1995	279.88	303	346.06	1,263	275.07	
14 th Dec 1995	280.31	322	347.00	1,155	274.77	
17 th Nov 1999	280.91	225	344.31	1,620	275.95	
18 th Nov 1999	281.28	241	344.74	1,528	275.72	

These flood events were modelled in steady state to develop modelled rating curves at the two gauging sites. A composite inflow was added to the model at the Cardrona confluence to compensate for the inflow from the Cardrona River (which is not included in the model). The model was calibrated by adjusting the channel and floodplain roughness coefficients, with the final modelled rating curves compared to the NIWA curves shown on Figure 7 and Figure 8.

It should be noted that only the current NIWA rating curves have been used in the model calibration. These curves are valid for all three of the calibration events.

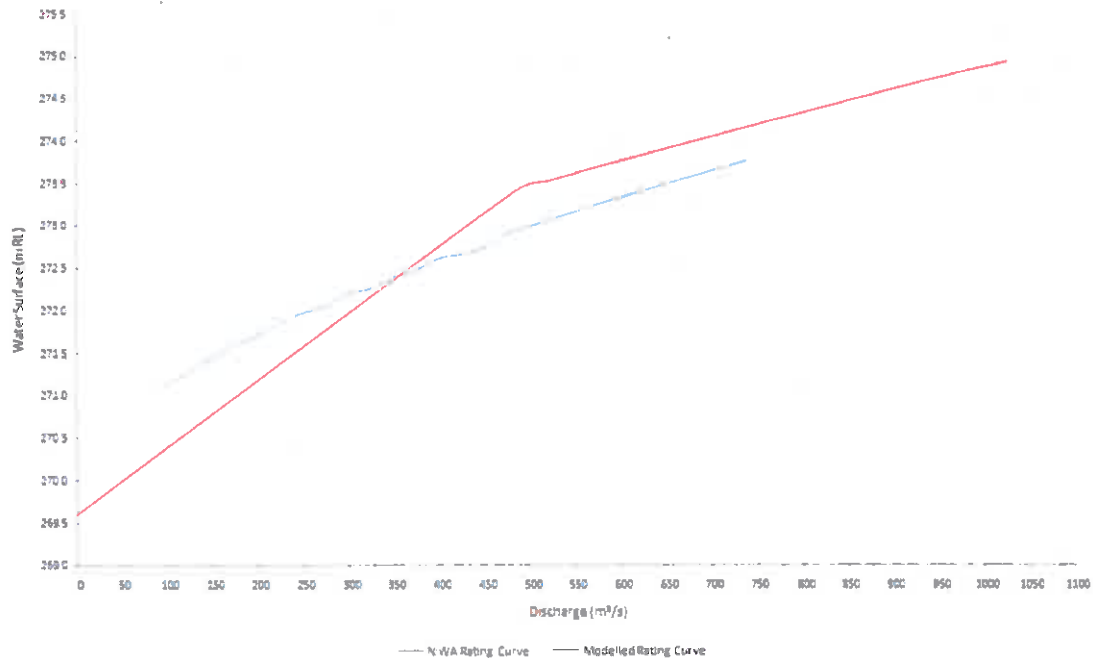
Figure 7 Model Calibration Results – Hawea @ Camp Hill Gauge



As shown on Figure 7, it was not possible to achieve a good calibration against the Camp Hill Road gauge, with a water level difference of over 1.50m at the high end of the NIWA rating curve. This is due to the lack of channel survey for the Hawea River, with the LiDAR survey not giving any elevations below the water line. The minimum elevation of the LiDAR cross section is RL 311.70m, however the minimum elevation on the NIWA rating curve is RL 310.40m with a rated discharge of 60m³/s below the minimum LiDAR elevation.

The calibration against the Cardrona gauge gave a better result, however the model is under-predicting water levels at the lower end of the rating curve and over-predicting at the higher end. The difference in water levels is 600mm at the high end of the rating curve.

Figure 8 Model Calibration Results – Clutha @ Cardrona Gauge



Although the model calibration has not given a satisfactory result, it was decided not to change the roughness coefficients significantly in order to improve the calibration. Based on the observed channel and floodplain roughness at the two gauge sites, the roughness values are of the right order.

The peak gauged flow at Camp Hill Road is only 1% of the rainy day breach discharge. At the Clutha @ Cardrona gauge the peak gauged flow is only 12% of the rainy day breach discharge. At both gauges the peak rainy day breach water level is approximately 17m higher than the top of the gauge staff. Although a calibration of the model has been attempted, the differences between the gauged and modelled rating curves are not considered significant. Instead the influence of roughness coefficient was assessed during the sensitivity analysis (see Section 5.1.4).

4.3.1 PMF (No Breach) Event

The rainy day breach model uses the 72 hour probable maximum flood hydrograph as the inflow to Lake Hawea. In their PMF update report Opus (2000) estimated a peak inflow to Lake Hawea of $5,440\text{m}^3/\text{s}$ and a peak lake level of RL 350.37m. To confirm the peak lake level (at which time the dam failure will be initiated in HEC-RAS) the PMF event has been modelled without any breach.

For this model run the PMF inflow hydrograph was scaled from Figure 5.15 (page 56) of the Opus report. When the hydrograph was routed through Lake Hawea using the stage-storage and discharge curves shown on Figure 4 the maximum water level in Lake Hawea was RL 350.24m. This is a difference of -130mm against the Opus report. Given that this lake level falls within the extrapolated section of the Lake Hawea storage curve, this is not considered to be a significant difference.

A restart file was created for the PMF event at the time of peak lake level. This restart file was then used to define the initial conditions for the rainy day breach model runs. This reduced model run times and file sizes without impacting on model accuracy.

5.0 Sensitivity Analyses

Because of the inherent uncertainties involved in dam break modelling, the sensitivity of the model has been tested against several parameters.

5.1.1 Breach Formation Time

The base case model runs used a breach formation time of 1.0 hour for both sunny and rainy day failures. Three sensitivity analyses were undertaken for each failure event, using formation times of 30mins, 2 hours and 3 hours. The peak discharges are shown in Table 7.

Table 7 Impact of Breach Formation Time on Peak Discharge

Breach Formation Time (hrs)	Sunny Day Event		Rainy Day Event	
	Peak Discharge (m ³ /s)	Change from Base Case (%)	Peak Discharge (m ³ /s)	Change from Base Case (%)
0.5	14,472	-0.14	21,359	+0.33
1.0 (base case)	14,492	-	21,289	-
2.0	14,384	-0.75	21,114	-0.82
3.0	14,267	-1.55	20,881	-1.92

As the table shows, changing the breach formation time parameter has no significant impact on the peak discharge from Lake Hawea, with a maximum change of less than 2%. This is likely due to the very large storage volume in the lake; the modelled hydrographs show it would take over a week to fully draw down the lake and nearly five hours from full breach formation to draw the lake down by 1m.

5.1.2 Breach Progression Type

HEC-RAS has the functionality to model breach progression as either a linear or sinusoidal process, based on a user specified time to peak (breach formation time). For the base case model runs the breach progression was modelled as a linear process with a sinusoidal progression modelled as a sensitivity analysis.

Table 8 shows the breach progression type has no impact on the peak discharge, with a maximum change of less than 1% for the sunny day event. As with the breach formation time, this is likely due to the large storage volume and the amount of time required to draw the lake down.

Table 8 Impact of Breach Progression Type on Peak Discharge

Breach Progression Type	Sunny Day Event		Rainy Day Event	
	Peak Discharge (m ³ /s)	Change (%)	Peak Discharge (m ³ /s)	Change (%)
Linear Progression (T _b 1hr)	14,492	-	21,289	-
Sinusoidal Progression (T _b 1hr)	14,358	-0.92	21,246	-0.20

5.1.3 Initial Lake Levels

The base case models use an initial water level of RL 277.20m for Lake Wanaka. While this is considered appropriate for the sunny day event, a sensitivity analysis has been done for the rainy day event using a higher initial level of RL 280.87m. This level corresponds to a 100yr ARI event based on an analysis of mean daily lake level records from 1933-2011 (data supplied by NIWA).

The Lake Wanaka PMF level (RL 282.84m, Opus 2000) was not used as the joint probability of PMF events in both the Hawea and Wanaka catchments combined with a failure of Hawea Dam would be extremely low. This combination of events was considered overly conservative, so the 100yr ARI lake level was used.

The effect of a higher initial lake level is a 200m³/s increase in the peak flow from Albert Town into Lake Wanaka, peaking at 6,730m³/s. The maximum lake level increases to RL 283.45m, a rise of 2.58m. This result is the opposite of what would be expected; the higher lake level would be expected to result in a smaller peak flow from Albert Town. However, a lake level of RL 280.87m gives a discharge of 1,100m³/s from Lake Wanaka meaning water levels in Albert Town are already elevated prior to arrival of the dam breach flood wave. This in turn increases the hydraulic fall between Albert Town and Lake Wanaka from 2.36m to 3.78m (at the time the flood wave arrives in Albert Town), resulting in a higher peak discharge into Lake Wanaka.

The higher initial water level in Lake Wanaka significantly increases the flood extent, number of properties inundated and the population at risk. This is discussed in Section 6.3.

5.1.4 Hydraulic Roughness

The sensitivity of the model to the hydraulic roughness coefficient was tested by increasing the floodplain roughness from the base case values (0.044 on the Clutha reaches and 0.048 on the Hawea) to 0.060 throughout the model, an increase of 25-36%.

The roughness sensitivity was necessary because the model calibration did not give satisfactory results and also to consider potential effects of debris transport. During the site visit it was observed that many sections of the Clutha and Hawea rivers have heavily wooded banks that would have a higher roughness coefficient than 0.048, but the calibration results suggested a lower coefficient was required. Bearing in mind the modelled flow depths (which in many areas would result in trees being submerged or washed away) a value of 0.060 was used.

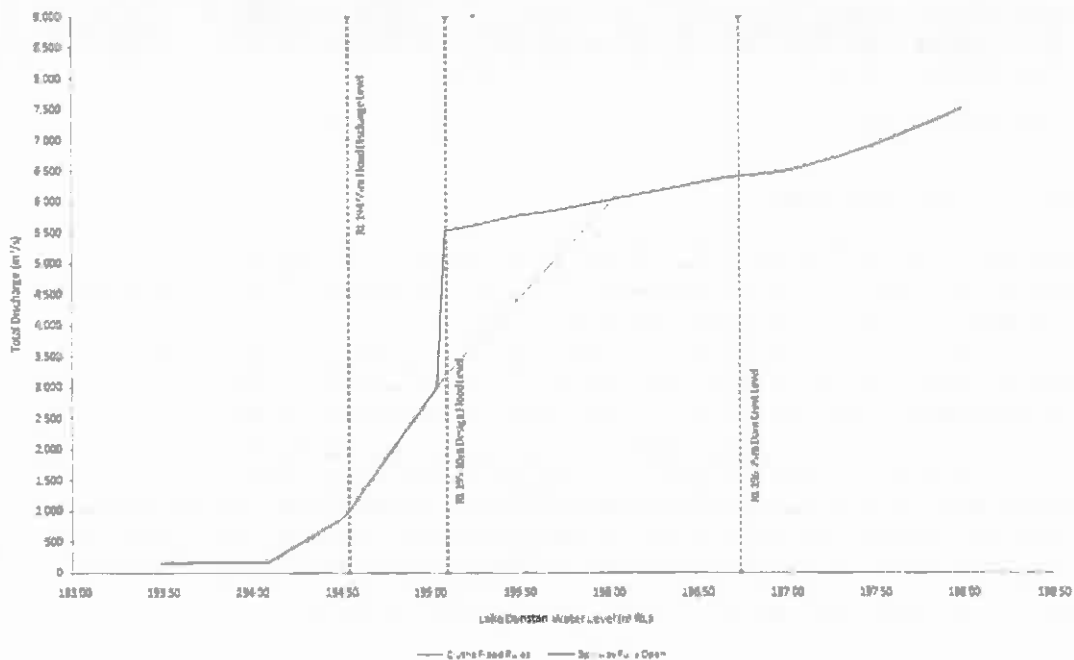
A full tabulation of the sensitivity results is shown in Appendix A. On the Hawea and Clutha Rivers the increased roughness gives a peak flow reduction of 4.0 – 4.4% compared to the base case runs, while the peak water depth increases by an average 4% (with a range between -2.2% and +10.9%). The most significant water level changes occur around the Church Road bridge at Luggate and on the upper Clutha River near the Lake Wanaka outlet.

5.1.5 Clyde Dam Discharge

The base case sunny and rainy dam breach models have been run using the stage-discharge relationship given in Contact's Clutha Flood Rules document, as shown in Figure 9. An alternative scenario has also been modelled, which uses Contact's flood rules while the Lake Dunstan water level is below RL 195.10m (which equates to the design flood level); once lake levels start rising above this level the spillway gates are opened fully and the machines are shut down. This stage-discharge curve is also shown on Figure 9. This alternative scenario is similar to that modelled in the 1990 study.

The objective of these model runs was to determine if overtopping of the Clyde Dam could be avoided if the peak discharge was increased earlier during the flood event. As the flood wave would have a travel time of seven hours from Lake Hawea to the upstream extent of Lake Dunstan, plus the breach formation time (in this case 1 hour) there would be plenty of warning time to start increasing discharge from the Clyde Dam and lowering the level of Lake Dunstan.

Figure 9 Modelled Stage-Discharge Curves for Clyde Dam



The peak discharge from the Clyde Dam and water level in Lake Dunstan are similar under both discharge scenarios. The Clyde Dam would be overtopped in both a sunny and rainy day event regardless of the spillway operation, with the depth of overtopping reduced by only 0.07m and 0.02m for the sunny and rainy day events respectively, as shown in Figure 11.

Table 9 Lake Dunstan Water Levels and Clyde Dam Overtopping Durations

	Sunny Day Event			Rainy Day Event		
	Peak Water Level (m RL)	Overtopping Duration (hrs)	Peak Discharge (m ³ /s)	Peak Water Level (m RL)	Overtopping Duration (hrs)	Peak Discharge (m ³ /s)
Clutha Flood Rules	197.74	16	7,195	199.59	26	9,585
Spillway Fully Open	197.67	15	7,105	199.57	26	9,527
Change	-2%	-6%	-1%	-	-	-

Note: The change in peak water level is relative to the minimum operating level of RL 193.50m

Figure 10 Lake Dunstan Time Series – Sunny Day Failure

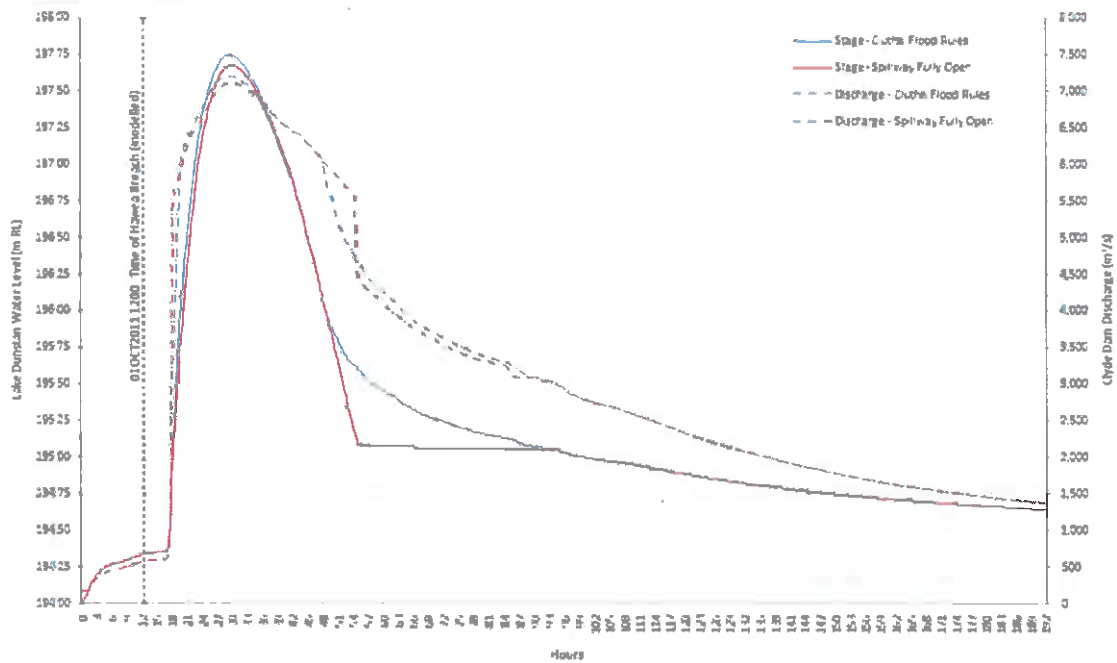
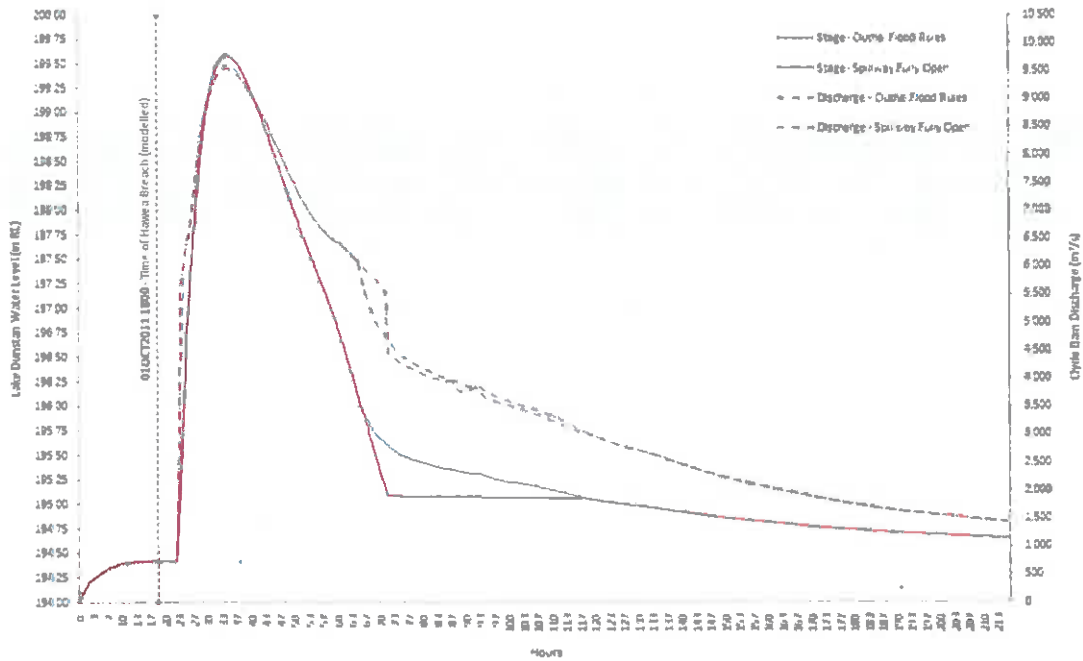


Figure 11 Lake Dunstan Time Series – Rainy Day Failure



6.0 Damage Assessment

The modelling results for the sunny and rainy day breach models are discussed in the following sections, with the flood inundation maps shown in Appendix D. It should be noted that travel times refer to the time taken for the flood wave to travel from the Hawea Dam to a particular location. With a one hour breach formation time, the warning time (the temporal lag between breach initiation and arrival of the flood wave) at any particular location will be the travel time plus one hour.

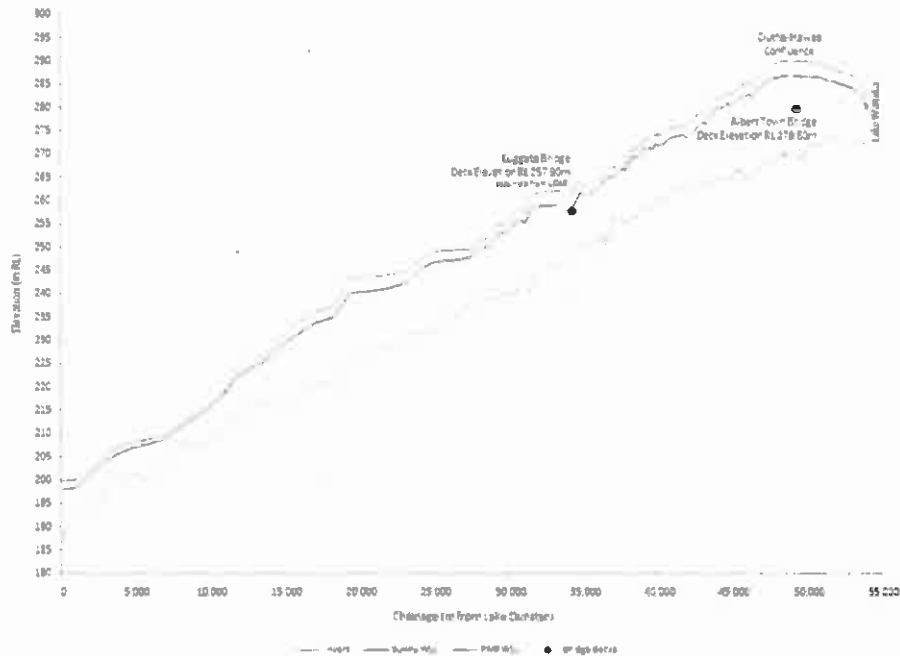
The Hawea Dam is a well monitored structure and it is assumed that flood warnings could be issued within 15 minutes of initiation of a sunny day breach (this assumes there would be staff on site following the hypothetical event that initiates the failure). For a rainy day breach there is a lag time of several days between when the start of the flood event and when the lake level exceeds the dam core, so it is again assumed that early flood warnings could be issued.

6.1 Hawea River

Although the Hawea River is deeply incised in its upper reaches, the depth of flood water following a breach means there would be inundation onto the Hawea Flats downstream of the Hawea oxidation ponds.

The only road crossing of the Hawea River is at Camp Hill Road, 5.0km downstream of Hawea Dam. The flood wave would reach the bridge 1.40 hours after initiation of the breach (40mins after maximum breach formation). As shown on Figure 12 the bridge would be submerged under 10m of water. With the increased local velocities through the bridge site (the bridge is at a constriction in the river giving average velocities of 11m/s) it is likely that the bridge would be destroyed.

Figure 13 Water Surface Profiles, Clutha River



6.3 Lake Wanaka

With an initial water level of RL 277.20m in Lake Wanaka and a peak flood level of RL 286.65m (for the sunny day event) in Albert Town, the normal flow direction would be reversed upstream of Albert Town. Water would spill into Lake Wanaka, which would experience a water level rise of 2.02m for the sunny day event and 2.98m for the rainy day event. Between Lake Wanaka and Albert Town the Clutha River is deeply incised with steep banks constraining the flood extent.

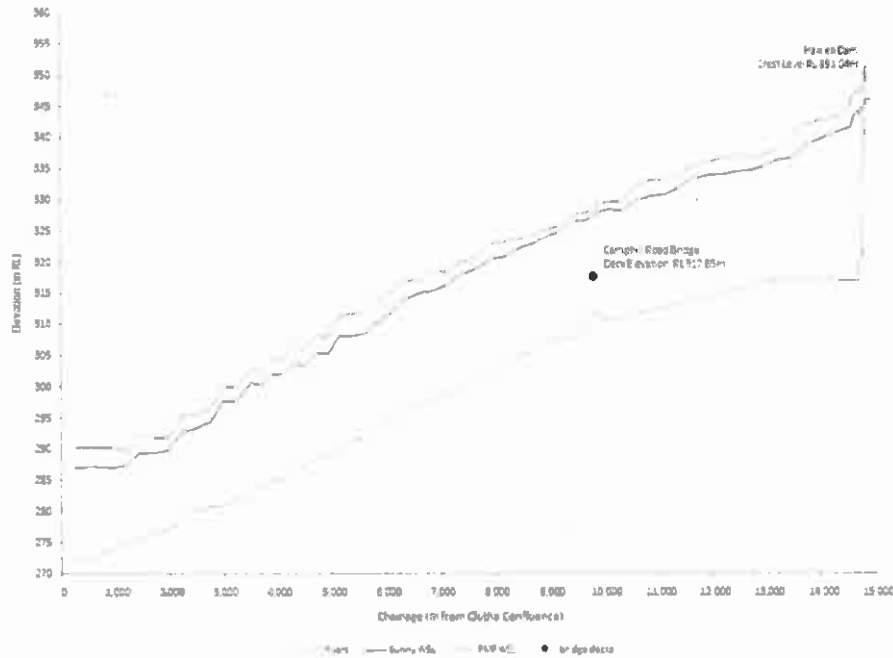
Lake Wanaka has a surface area of approximately 193km² and this large storage capacity means the lake would act as a flood detention basin following a breach of Hawea Dam. The peak inflow to the lake is 4,076m³/s during a sunny day event and 6,500m³/s during a rainy day event. The large storage volume means the peak lake level would not occur for 3 days after a Hawea failure.

When flood levels in the Clutha begin to drop, Lake Wanaka would then discharge the flood water back into the Clutha River. This increases the total duration of the flood event downstream of Albert Town, but significantly attenuates the peak discharge.

At Roys Bay the flood extents have been plotted using 1m interval contour data and aerial photography (see drawing 60221308-SD-08). This shows that during the sunny day event there would be minimal damage in Wanaka town (the peak water level in Lake Wanaka only raises to RL 279.22m). During a rainy day event the peak level would be slightly higher at RL 280.18m, causing surface flooding across Ardmore Street and inundation of three properties on the corner of Ardmore and Dungarvon Streets. Outside of Roys Bay the flood mapping has used 20m interval contour data and 1:25,000 topographic maps; these only provide a large scale indication of the potential flood extent at other areas around the lake shore.

The sensitivity analyses show that if the lake had a higher initial water level (close to the 100 year ARI of RL 280.87m) there would be extensive inundation and damage through Wanaka town. The lake level rises by 2.58m to a peak of RL 283.45m. The flood extent (also shown on drawing 60221308-SD-01) shows a large part of the town centre would be inundated between Ardmore and Brownston Streets to the north-south and Helwick and Dungarvon Streets to the east-west. From the aerial photographs, approximately 60 buildings, including several hotels, numerous cafes and a supermarket would all be inundated. Pembroke Park and Wanaka Recreational Reserve would also be inundated. On the north side of the town, Lakeside Drive would be flooded between the

Figure 12 Water Surface Profiles, Hawea River



Downstream of the bridge there would be extensive inundation on the right bank. On the left bank Newcastle Road would be inundated for a length of 2.80km.

Around Horseshoe Bend the flood extent would be constrained by bluffs on both banks of the river, however there are several properties in this area that would all be flooded. The flood wave would reach these properties at Horseshoe Bend around 2.50 hours after initiation of the breach.

As the Hawea River approaches the confluence with the Clutha River the floodplain widens and the flood extent increases. There is a camp site and kayak slalom course located 1.50km upstream of the confluence. Peak flood depths at this location would be in the order of 14m with a travel time of 1.50 hours from the dam (2.50 hours after breach initiation).

There is a Department of Conservation (DOC) campsite located to the west of SH6 at the Hawea-Clutha confluence. The depth of inundation at the camp site would be in the region of 16m. The SH6 road would also be inundated for a distance of 800m on the northern approach to the SH6 Albert Town bridge.

6.2 Albert Town

Albert Town is located at the confluence of the Hawea and Clutha Rivers and would experience widespread inundation and damage following a failure of Hawea Dam. The low lying area around the confluence would be inundated and there would be negative flow up the Cardrona River.

The flood peak would arrive at Albert Town 2.5 hours after the initiation of a breach at Hawea with the peak water level occurring 1.5 hours later (4.0 hours after breach initiation).

With a peak water level of RL 286.62m during the sunny day event and RL 289.90m during the rainy day event the SH6 bridge would be overtopped and remain submerged for 48 hours. The flood water would start to overtop the bridge 3.5 hours after the breach initiation. Secondary flow would occur around the north (true left) of the bridge, however there is also the possibility of erosion and slope instability on the south abutment.

The northern and eastern areas of Albert Town would be inundated during both a sunny and rainy day event, however the central section of the town (immediately north of State Highway 6) would only be inundated during a rainy day event.

intersections of Ardmore and Beacon Point Roads, but no properties are within the flood extent. Approximately 30 other properties located between Eely Point and Beacon Point would also be inundated.

6.4 Upper Clutha River

Downstream of Albert Town the peak flood discharge would be attenuated by both the ponding through Albert Town and the negative flow upstream to Lake Wanaka. The peak discharge downstream of Albert Town would be 9,125m³/s and 12,830m³/s for the sunny and rainy day events respectively, giving attenuation of approximately 30% for both events.

Downstream of Albert Town the Clutha River is deeply incised and the flood wave would be contained within the channel. State Highway 8A crosses the Clutha River at Luggate, approximately 30km downstream of the Hawea Dam. The deck elevation of the bridge has been estimated from the LiDAR survey but would be submerged and likely destroyed in both a sunny and rainy day event (see Figure 13). The flood wave would arrive at the bridge 4.50 hours after initiation of the Hawea breach.

The town of Luggate is above the peak flood level and is not at risk of inundation. Downstream of Luggate the Clutha River continues to be incised with only isolated areas of out-of-bank flooding. One of these areas is at Sandy Point (41km downstream of Hawea Dam) where the Aoturoa Farmstead is located. The farm buildings are located at the outer extent of the flood envelope and would only be inundated during a rainy day event, which would also result in some spill across SH6. The peak rainy day flood level at Aoturoa is RL 249.11m as the flood wave would arrive 5.50 hours after initiation of the Hawea breach.

Approaching the confluence with the Lindis River the channel starts to widen with the Clutha being braided downstream of the confluence. The channel expansion and resulting energy losses give significantly lower peak velocities and water depths downstream of the Lindis confluence. The flood would have an extent of 1.30 – 2.20km wide in these lower reaches of the Clutha. The farm properties at Willowbank and Bendigo would be inundated, as would a 1.60km length of SH8. The flood wave would arrive at these properties between 6.50 and 7.00 hours after initiation of the breach at Hawea.

6.5 Lake Dunstan and Clyde Dam

The peak water levels in Lake Dunstan are controlled by the operating rules for the Clyde Dam. The stage-discharge curve that would be used in the event of a major flood is shown on Figure 6. Because Lake Dunstan has been modelled as a storage node the reservoir is treated as a level pool, so the results do not show the hydraulic gradient through the lake that would occur in reality. The LiDAR survey does not cover Lake Dunstan and the flood envelopes have been created using 20m interval contours, so this modelling approach is considered to be acceptable.

During a sunny day event and assuming Contact's flood rules were followed and the initial lake level was RL 194.00m, Lake Dunstan would rise by 3.74m to a peak of RL 197.74m. The corresponding figures for a rainy day event are 5.59m and RL 199.59m. Because of the low resolution of the contour data used around Lake Dunstan it is difficult to state with confidence if any properties will be inundated along the lake front. However, the subdivision at the intersection of SH6 and Clarke Road (27km upstream of the Clyde Dam) is within both the sunny and rainy day flood envelopes, as is the Pisa Range Cromwell Hotel.

The flood envelopes also show some minor flooding around Cromwell during the rainy day event, but more detailed contour information is required to confirm this. The extent shown around Cromwell is not considered realistic as Cromwell has an elevation of RL 210m, putting it 10m clear of the peak lake level.

State Highway 6 on the western side of Lake Dunstan is within both the sunny and rainy day flood envelopes between Clarke Road and Lowburn with further isolated inundation south of Lowburn.

Downstream of Cromwell the flood waters would be contained within the Cromwell gorge. Some minor inundation of SH8 may occur.

The peak lake level would occur 17-18 hours after initiation of the Hawea breach.

The crest level of the Clyde Dam is RL 196.75m. As shown on Figure 10 and Figure 11, if the dam were operated using the existing Clutha flood rules the structure would be overtopped by 0.72m during a sunny day event and 2.84m during a rainy day event. These are the maximum modelled water depths above the crest, although the total duration that the dam would be overtopped is 26hrs for a rainy day event and 16hrs for a sunny day event. The dam has been designed to withstand overtopping.

6.6 Population at Risk

The population at risk (PAR) from a hypothetical sunny or rainy day failure of Hawea Dam has been estimated by exporting the flood inundation extents in KMZ format and using Google Earth to determine the number of properties lying within the flood boundary. The number of inundated properties in each area is shown in Table 10. The rainy day figures are for an initial level of RL 280.87m in Lake Wanaka.

Table 10 Number of Inundated Properties

	Properties	Hotels	Camp Sites
Sunny Day Event			
Hawea River	18	-	1
Lake Wanaka	-	-	1
Clutha u/s Albert Town	-	-	-
Albert Town	158	2	1
Clutha d/s Albert Town	4	-	-
Lake Dunstan	55	1	-
Sunny Day Total	235	3	3
Rainy Day Event			
Hawea River	22	-	1
Lake Wanaka	95	3	1
Clutha u/s Albert Town	-	-	-
Albert Town	230	2	1
Clutha d/s Albert Town	5	-	-
Lake Dunstan	68	1	-
Rainy Day Total	420	3	3

The average number of people per dwelling has been estimated at 2.3, based on NZ Government census data for Wanaka and Hawea. Campsite populations have been estimated using 2.5 people per tent site with 180 sites per campground. Hotel populations have been estimated using occupancy of 50 people per hotel. The campsite occupancy figures appear conservative as they assume that a Hawea breach would occur during the peak summer period, however given the three largest floods on record in the Clutha have occurred between November and January, the assumption is not unreasonable.

The above figures give a total PAR of 2,040 for the sunny day event and 2,280 for the rainy day event. If a rainy day event were to occur in coincidence with a high water level in Lake Wanaka, the PAR could increase to 2,620.

6.7 Comparison with the 1990 Study

A short comparison of results against the 1990 study is included in Table 11. These results are for the sunny day failure only, assuming a failure time of 3 hours and that the Clyde Dam spillway gates are open fully when Lake Dunstan reaches RL 195.10m.

Table 11 Comparison against 1990 Results

	1990 Study	2011 Study
Peak outflow from Hawea (m ³ /s)	12,300	14,267
Peak water level Albert Town (m RL)	284.50	286.65
Peak water level Lake Wanaka (m RL)	280.80	279.22

	1990 Study	2011 Study
Peak inflow to Lake Wanaka (m ³ /s)	6,510	4,076
Peak discharge downstream of Albert Town (m ³ /s)	5,360	9,139
Peak water level Lake Dunstan (m RL)	195.90	197.67
Peak inflow to Lake Dunstan (m ³ /s)	4,580	8,906

The results show that peak discharge and water level estimates are generally higher in this study than in the 1990 study, the exception being Lake Wanaka. Reasons for the differences in results could include the modelling methodology, topographic data used (i.e. cross sections), and extrapolation of lake storage curves.

Although the 1990 study was a very comprehensive report, the results of this study are considered to be more accurate because of the better quality ground survey data used and the improvements in modelling software that have occurred over the last 20 years.

7.0 Conclusion

Failures of earth fill dams such as the lake Hawea Control Dam are extremely rare but have occurred in the past, either as a result of internal erosion or overtopping. In this study a failure resulting from internal erosion was assumed for both the sunny and rainy day events. The sunny day event considered an earthquake induced failure during normal operating conditions, with the rainy day event considering failure following high lake levels and internal erosion resulting from seepage above the dam core.

With a maximum PMF lake level of RL 350.24m an overtopping failure was not considered, particularly as the Gladstone Gap stopbank could be breached in the event of lake levels approaching RL 351.00m. Consequences resulting from a breach of the Gladstone Gap were not considered in this study.

The peak discharge from the breach is estimated to be 14,500m³/s for the sunny day event and 21,300m³/s for the rainy day event.

There would be extensive damage through Albert Town and all three bridges across the Hawea and Clutha rivers would likely be destroyed, leaving the SH8 bridge at Cromwell as the only remaining crossing of the Clutha. Depending on the initial water levels in Lake Wanaka there could also be extensive damage through Wanaka town.

The Hawea and Clutha Rivers are entrenched in old fluvio-glacial deposits and severe bed and bank erosion could be expected, although the locations and extent of such erosion have not been considered in this study. The banks of the rivers are heavily wooded and large debris flows (comprising trees, eroded bank material, material from the Hawea Dam etc) could also be expected. Some of this material could be deposited around Albert Town where velocities would be lower due to ponding.

The population at risk following a breach of Hawea Dam is estimated to be in the range of 2,040 to 2,620 people, mostly centred in Albert Town and Wanaka. The actual PAR would depend on the time of the year, with the figure being highest in the summer months, particularly around the Christmas period. The travel time of the flood wave is estimated to be two hours to Albert Town, with a one hour breach formation time giving a total warning time of three hours (assuming detection on initiation of the breach). Peak water levels in Lake Wanaka would occur two days after a breach of Hawea, giving sufficient time for Wanaka town to be evacuated if necessary.

8.0 References

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Works Consultancy Services (1990), Hawea Lake Control Dam; Report on Dam Break Analysis

Appendix A

Tabular Results

Appendix A Tabular Results

Table A1 – Base case model results for the sunny and rainy day events

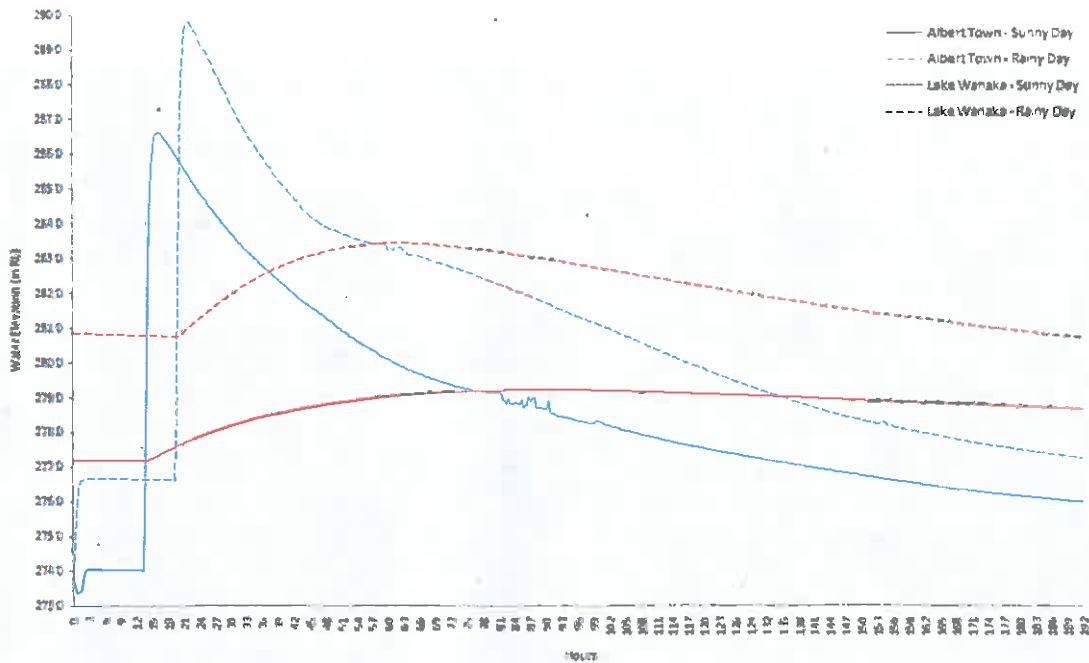
Table A2 – Sensitivity results with increased hydraulic roughness for sunny and rainy day events

Appendix B

Model Hydrographs

Appendix B Model Hydrographs

Figure B1 Stage Hydrographs at Albert Town and Lake Wanaka



Note – the rainy day hydrographs shown on this figure are for an initial Lake Wanaka level of RL 280.87m

Figure B2 Discharge Hydrographs at Selected Locations (Sunny Day Event)

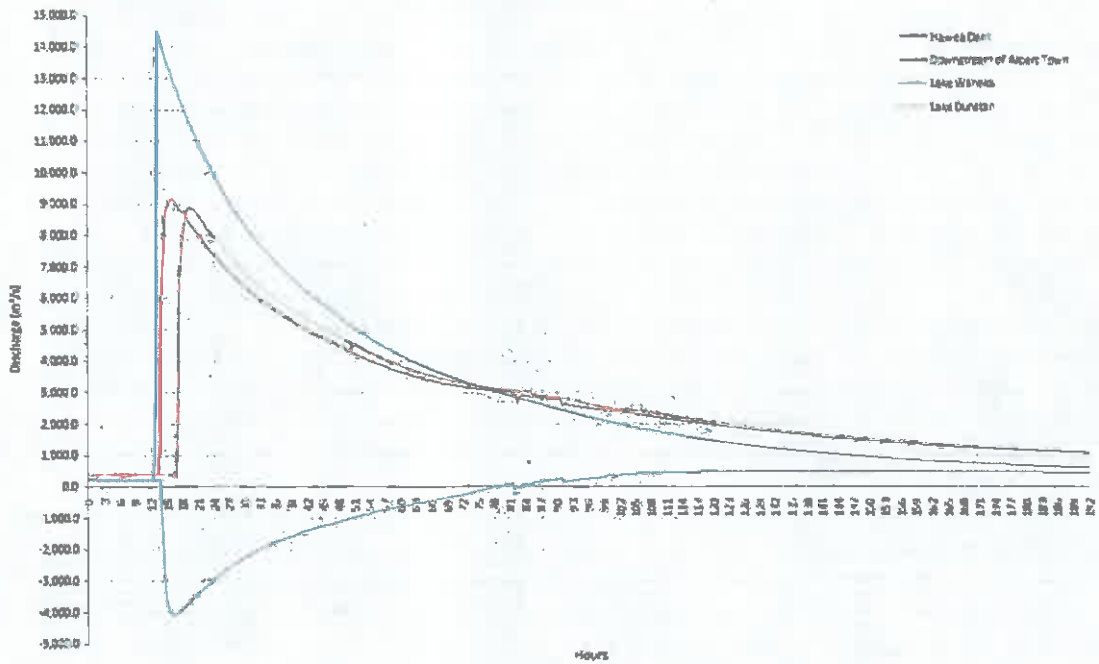


Figure B3 Profile of Peak Discharge in the Clutha River

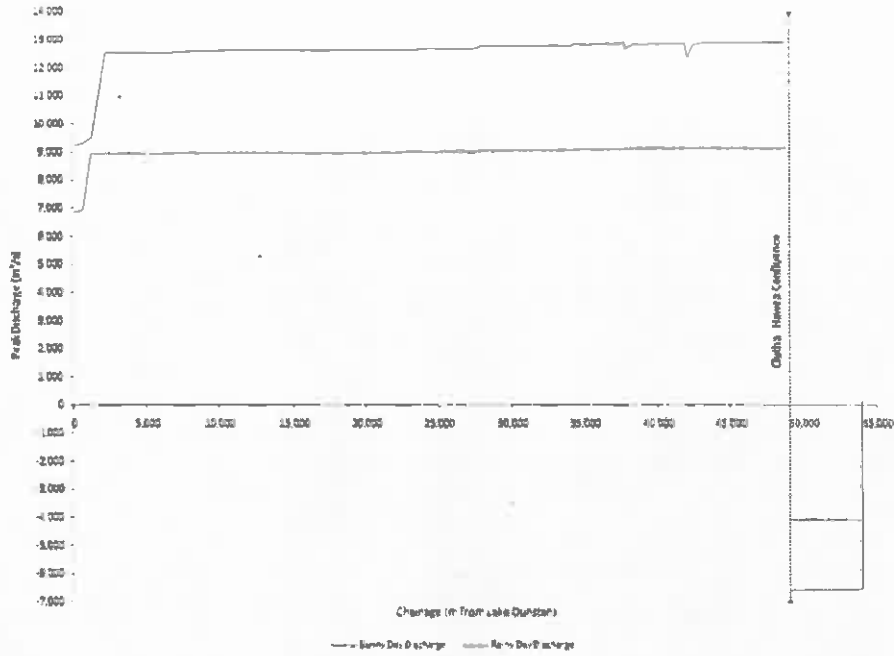
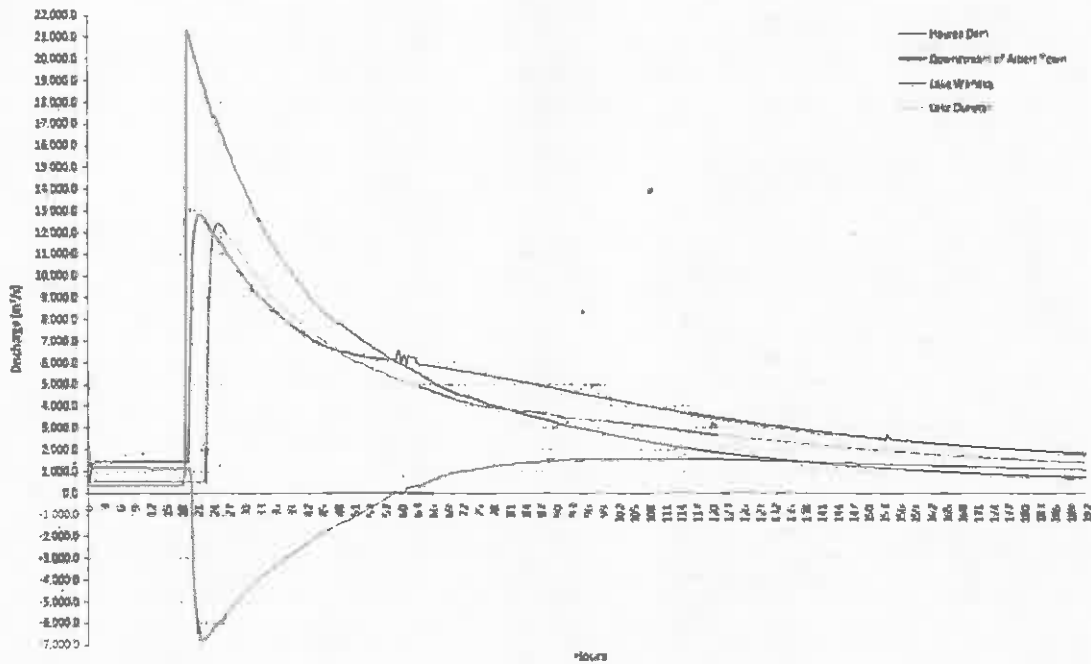


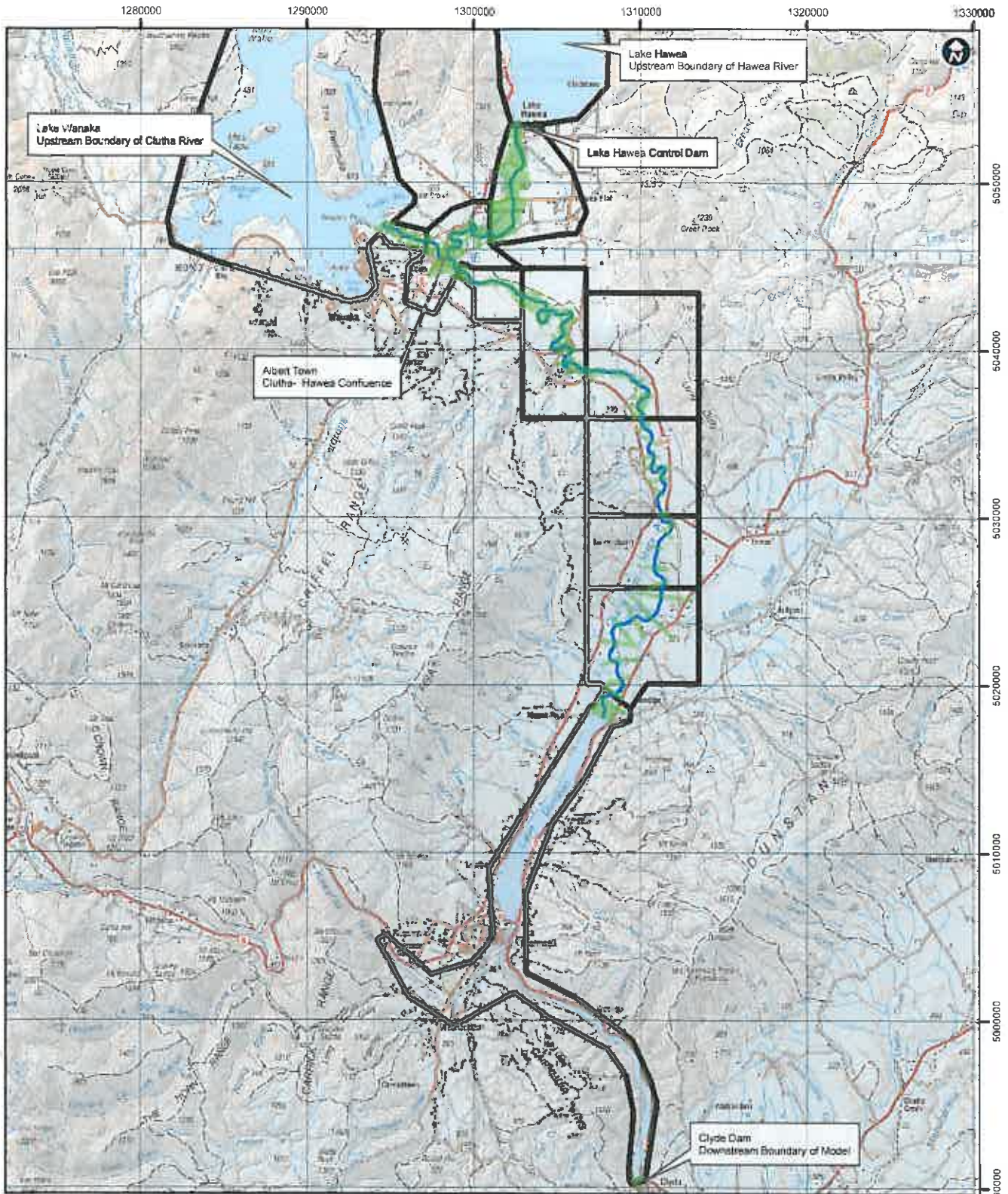
Figure B4 Discharge Hydrographs at Selected Locations (Rainy Day Event)



Note – rainy day hydrographs shown on this figure are for an initial Lake Wanaka level of RL 280.87m

Appendix C

Model Extents



Notes:

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Map features depicted in terms of NZTM projection
 Date Sources
 NZ Topographic Datasets, LINZ 2005
 NZ CRS Database LINZ 2007

Legend:

- Model Cross Sections
- River Centerlines
- DTM and Model Extents



Ref No: **Figure C.1**

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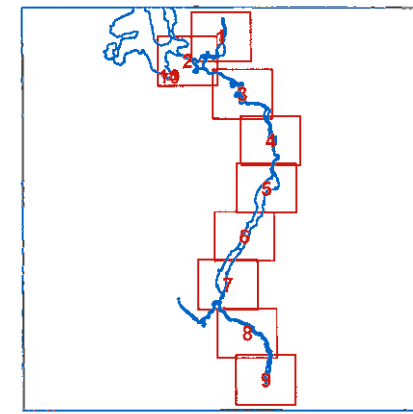
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Rev	Description	Date	By	Check	Appr
01	Issue for Review	12/12/2009	J. Smith	M. Jones	P. Brown
02	Revised	15/12/2009	J. Smith	M. Jones	P. Brown
03	Final	18/12/2009	J. Smith	M. Jones	P. Brown

LAKE HAWEA CONTROL DAM
Dam Break Update Study
 Hydraulic Model Extents

Appendix D

Flood Inundation Maps



Key Map:

Legend

- Sunny Day Flood Extent
- Flood Wave Travel Time
- Map Sheets

Notes:
 The inundated area shown is based on an hypothetical sunny day failure of the Hawea control structure.
 Time of peak discharge and water level have been rounded to the nearest 20mm.
 Flood extents have been derived using LIDAR data supplied by Contact Energy with a vertical accuracy of +/- 250mm.
 All levels are to MSL Duneidun Datum.
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Client: **Contact Energy**
 Project: **Lake Hawea Control Dam Dam Break Update Study**

Sheet No: 1 | Project No: 60221308 | Issue Rev

Title: **Sunny Day Failure Inundation Map Sheet 1 of 10**

Scale: 1:30,000 (A3 size)

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PJC	PJC	Approved	GB	28 Sep 11
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JOIN WITH SHEET 2



Lake Wanaka 19,900m from Hawea Dam

	Sunny Day	Rainy Day
Time to Max. Discharge (hrs)	4.00	3.40
Time to Max. Water Level (hrs)	75.00	43.00
Max. Discharge (m ³ /s)	4,075	6,775
Max. Water Level (m RL)	279.22	283.45
Max. Water Level Rise (m)	2.02	2.58

Albert Town Upstream of Hawea Confluence

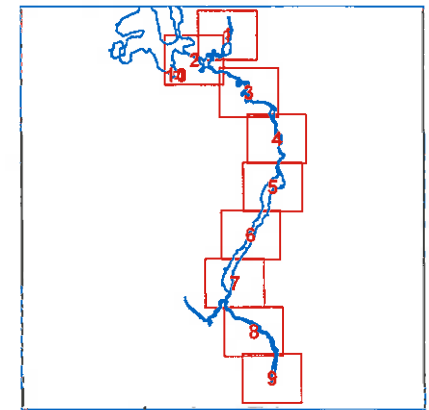
	Sunny Day	Rainy Day
Time to Max. Discharge (hrs)	3.00	3.00
Time to Max. Water Level (hrs)	4.00	3.40
Max. Discharge (m ³ /s)	4,380	6,950
Max. Water Level (m RL)	266.61	269.76
Max. Water Level Rise (m)	15.67	16.84

Hawea-Clutha Confluence 14,700m from Hawea Dam

	Sunny Day	Rainy Day
Time to Max. Discharge (hrs)	2.20	2.40
Time to Max. Water Level (hrs)	4.00	3.40
Max. Discharge (m ³ /s)	13,640	19,975
Max. Water Level (m RL)	287.03	290.09
Max. Water Level Rise (m)	15.48	18.54

Clutha River Downstream of Albert Town

	Sunny Day	Rainy Day
Time to Max. Discharge (hrs)	3.20	3.20
Time to Max. Water Level (hrs)	4.00	3.40
Max. Discharge (m ³ /s)	9,140	12,790
Max. Water Level (m RL)	286.64	289.81
Max. Water Level Rise (m)	15.64	18.81



Key Map:

- Legend**
- Sunny Day Flood Extent
 - Flood Wave Travel Time
 - Map Sheets

Notes
 The inundated area shown is based on an hypothetical sunny day failure of the Hawea control structure. Time of peak discharge and water level have been rounded to the nearest 20mins. Maximum water levels shown assume an initial water level in Lake Wanaka of RL 277.20m for the sunny day event and RL 280.87m for the rainy day event. Flood extents have been derived using LIDAR data supplied by Contact Energy with a vertical accuracy of +/- 250mm. All levels are to MSL Dunedin Datum. Background image sourced from LINZ. Crown Copyright Reserved.



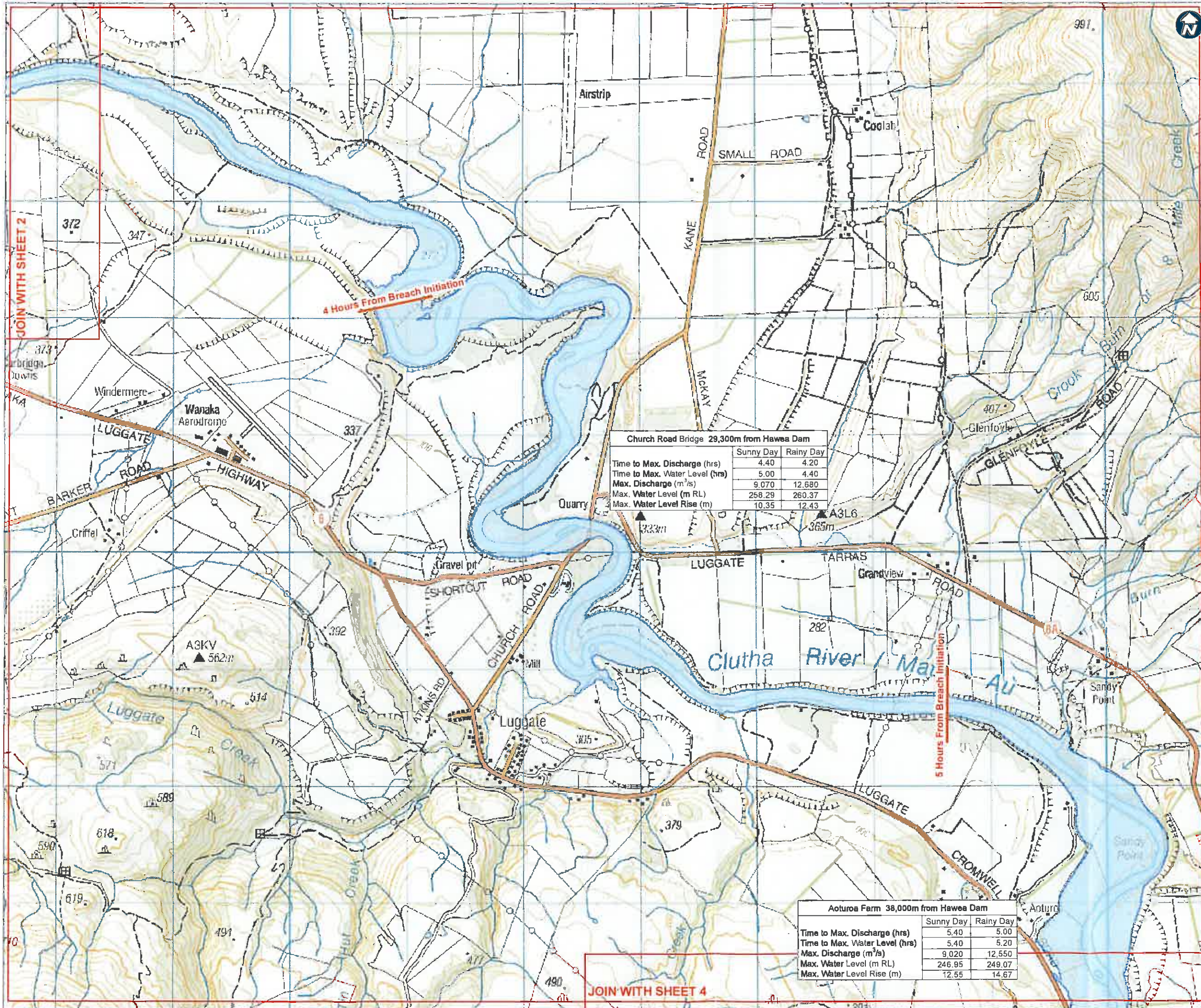
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Sheet No	Project No.	Issue	Rev
2	60221308		

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 Sheet 2 of 10

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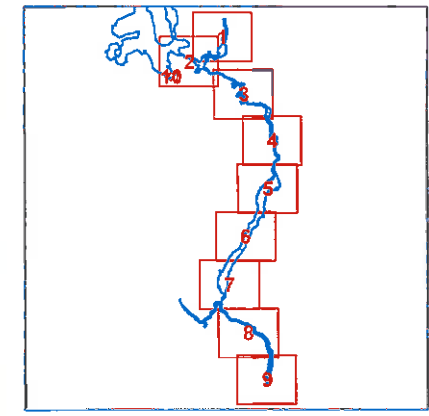
JOIN WITH SHEET 4

Church Road Bridge 29,300m from Hawea Dam

	Sunny Day	Rainy Day
Time to Max. Discharge (hrs)	4.40	4.20
Time to Max. Water Level (hrs)	5.00	4.40
Max. Discharge (m ³ /s)	9,070	12,680
Max. Water Level (m RL)	258.29	260.37
Max. Water Level Rise (m)	10.35	12.43

Aoturoa Farm 38,000m from Hawea Dam

	Sunny Day	Rainy Day
Time to Max. Discharge (hrs)	5.40	5.00
Time to Max. Water Level (hrs)	5.40	5.20
Max. Discharge (m ³ /s)	9,020	12,550
Max. Water Level (m RL)	246.95	249.07
Max. Water Level Rise (m)	12.55	14.67



Key Map:

- Legend
- Sunny Day Flood Extent
 - Flood Wave Travel Time
 - Map Sheets

Notes
 The inundated area shown is based on an hypothetical sunny day failure of the Hawea control structure.
 Time of peak discharge and water level have been rounded to the nearest 20mins
 Maximum water levels shown assume an initial water level in Lake Wanaka of RL 277.20m for the sunny day event and RL 260.87m for the rainy day event
 Flood extents have been derived using LIDAR data supplied by Contact Energy with a vertical accuracy of +/- 250mm
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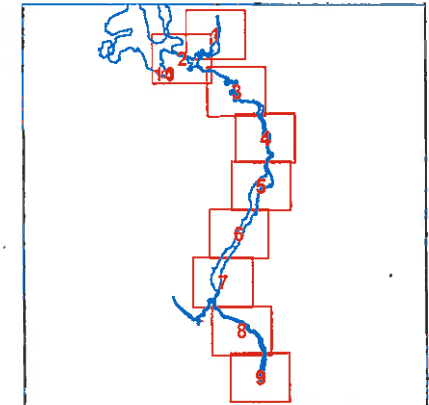
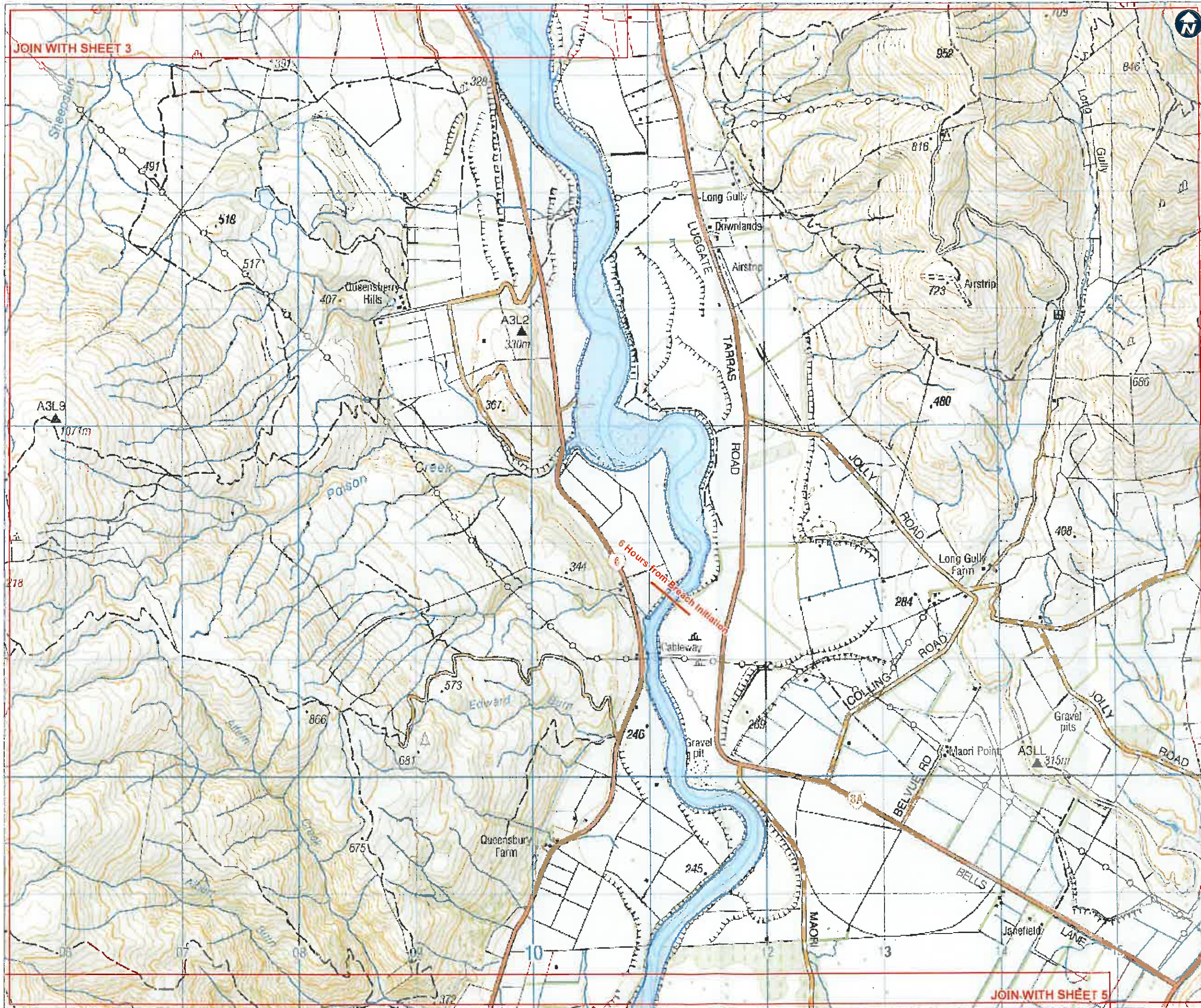


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		Issue	Rev
Title			
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Key Map:

- Legend**
- Sunny Day Flood Extent
 - Flood Wave Travel Time
 - Map Sheets

Notes
 The inundated area shown is based on an hypothetical sunny day failure of the Hawea control structure.
 Time of peak discharge and water level have been rounded to the nearest 20mins
 Maximum water levels shown assume an initial water level in Lake Wanaka of RL 277.20m for the sunny day event and RL 280.87m for the rainy day event
 Flood extents have been derived using LIDAR data supplied by Contact Energy with a vertical accuracy of +/- 250mm
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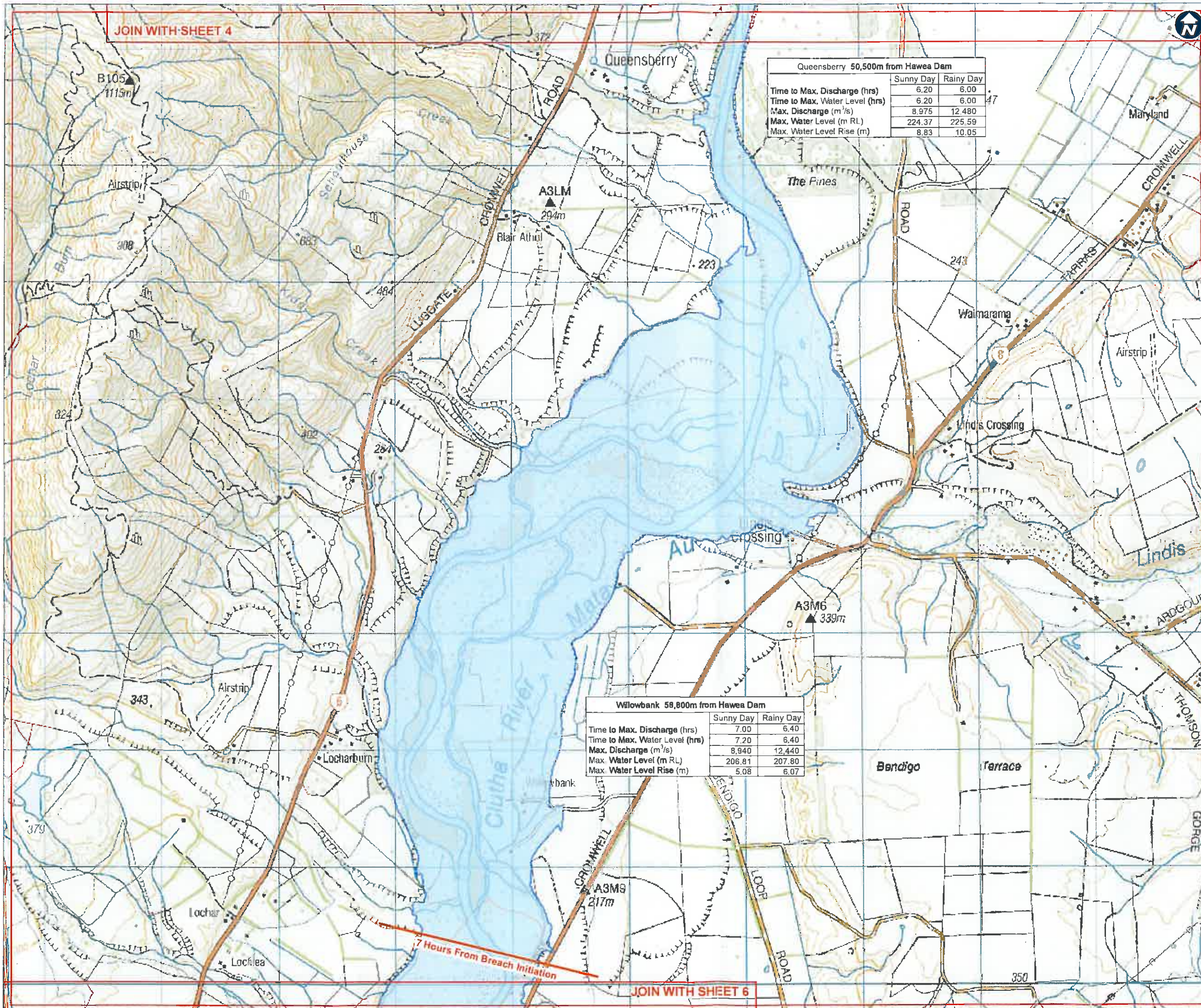
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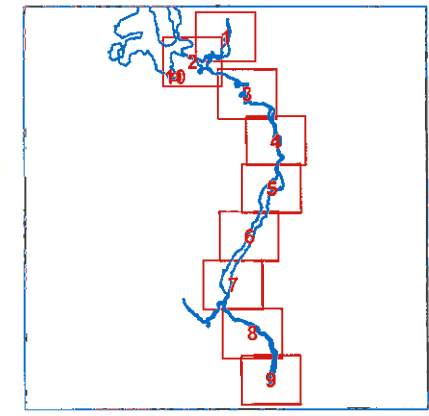
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Queensberry 50,500m from Hawea Dam		
	Sunny Day	Rainy Day
Time to Max. Discharge (hrs)	6.20	6.00
Time to Max. Water Level (hrs)	6.20	6.00
Max. Discharge (m ³ /s)	8,975	12,480
Max. Water Level (m RL)	224.37	225.59
Max. Water Level Rise (m)	8.83	10.05

Willowbank 58,800m from Hawea Dam		
	Sunny Day	Rainy Day
Time to Max. Discharge (hrs)	7.00	6.40
Time to Max. Water Level (hrs)	7.20	6.40
Max. Discharge (m ³ /s)	8,940	12,440
Max. Water Level (m RL)	206.81	207.80
Max. Water Level Rise (m)	5.08	6.07

JOIN WITH SHEET 6



Key Map:

- Legend
- Sunny Day Flood Extent
 - Flood Wave Travel Time
 - Map Sheets

Notes
 The inundated area shown is based on an hypothetical sunny day failure of the Hawea control structure.
 Time of peak discharge and water level have been rounded to the nearest 20mins.
 Maximum water levels shown assume an initial water level in Lake Wanaka of RL 277.20m for the sunny day event and RL 280.87m for the rainy day event.
 Flood extents have been derived using LIDAR data supplied by Contact Energy with a vertical accuracy of +/- 250mm.
 All levels are to MSL, Dunedin Datum.
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Sheet No	5	Project No	60221308
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Sunny Day Failure Inundation Map Sheet 5 of 10			
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Design	Checked	Date
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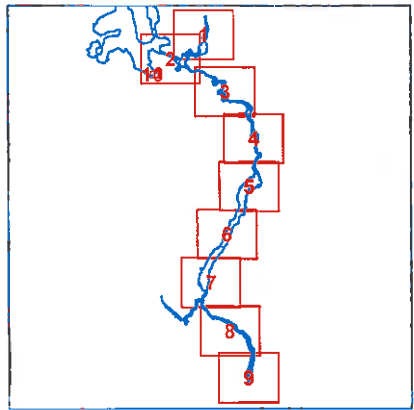


Bendigo 62,200m from Hawea Dam

	Sunny Day	Rainy Day
Time to Max. Discharge (hrs)	7.00	7.00
Time to Max. Water Level (hrs)	7.00	7.00
Max. Discharge (m ³ /s)	8,940	12,430
Max. Water Level (m RL)	201.59	202.67
Max. Water Level Rise (m)	7.62	8.70

Lake Dunstan

	Sunny Day	Rainy Day
Time to Max. Discharge (hrs)	7.20	7.00
Time to Max. Water Level (hrs)	12.00	16.40
Max. Discharge (m ³ /s)	7,195	9,625
Max. Water Level (m RL)	197.74	199.66
Max. Water Level Rise (m)	3.74	5.66



Key Map:

Legend

- Sunny Day Flood Extent
- Flood Wave Travel Time
- Map Sheets

Notes
 The inundated area shown is based on an hypothetical sunny day failure of the Hawea control structure.
 Time of peak discharge and water level have been rounded to the nearest 20mins.
 Maximum water levels shown assume an initial water level in Lake Wanaka of RL 277.20m for the sunny day event and RL 280.87m for the rainy day event.
 Maximum water levels for Lake Dunstan assume the Clyde Dam is operated as per Contact's Clutha flood rules.
 Flood extents at Lake Dunstan have been derived using contour data with a vertical interval of 20m, obtained from LINZ.
 All levels are to MSL Dunedin Datum.
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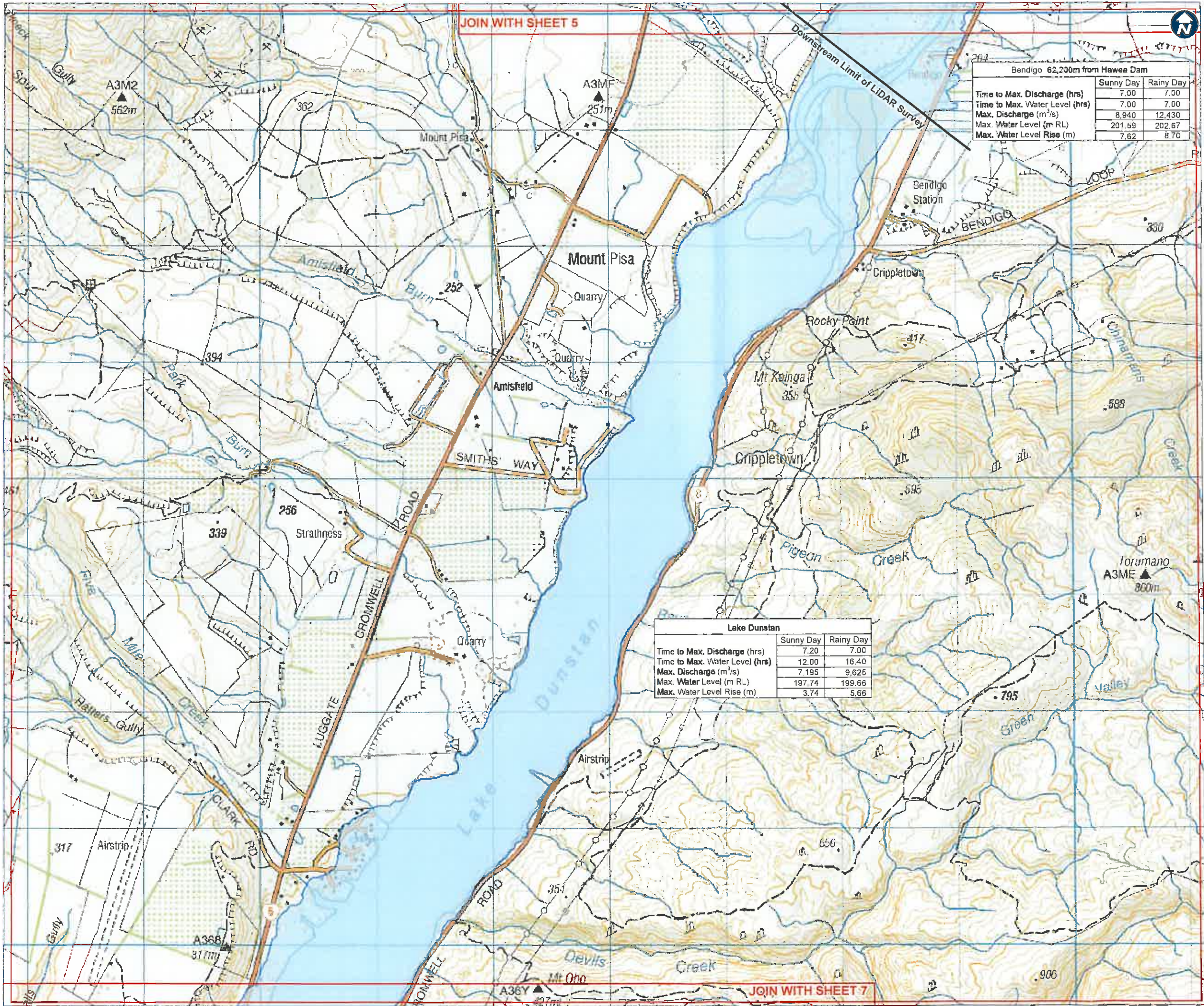
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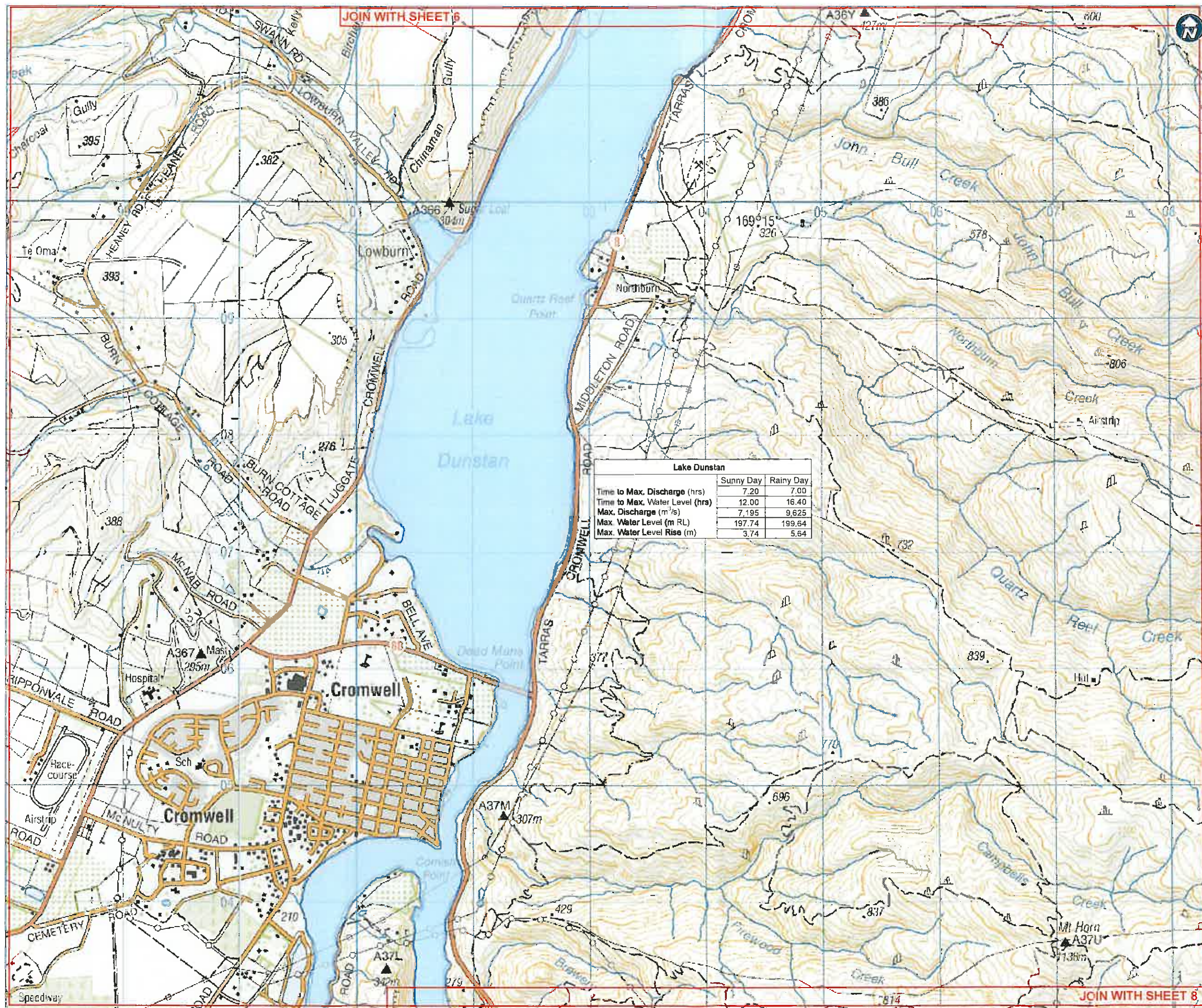
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**Sunny Day Failure Inundation Map
 Sheet 6 of 10**

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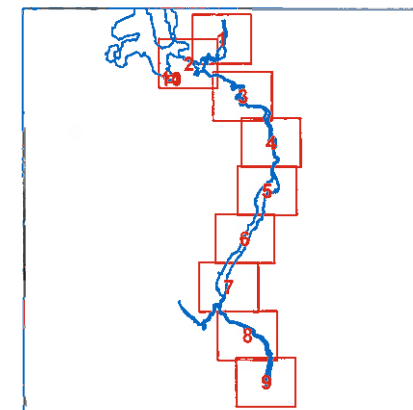


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JOIN WITH SHEET 6

JOIN WITH SHEET 8



Key Map:



Notes
 The inundated area shown is based on an hypothetical sunny day failure of the Hawea control structure. Time of peak discharge and water level have been rounded to the nearest 20mins.
 Maximum water levels shown assume an initial water level in Lake Wanaka of RL 277.20m for the sunny day event and RL 280.87m for the rainy day event.
 Maximum water levels for Lake Dunstan assume the Clyde Dam is operated as per Contact's Clutha flood rules.
 Flood extents at Lake Dunstan have been derived using contour data with a vertical interval of 20m, obtained from LINZ. All levels are to MSL Dunedin Datum.
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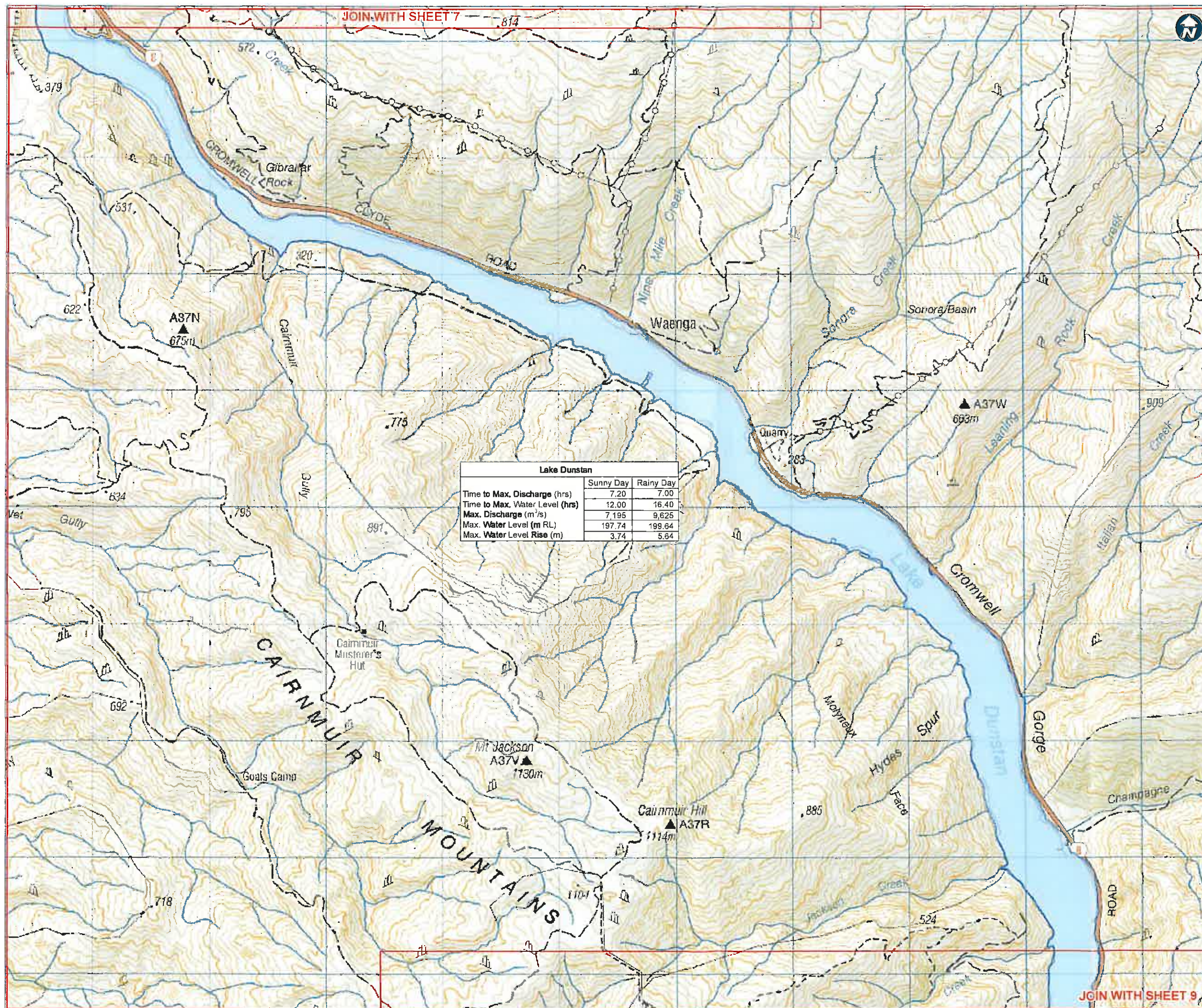
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Title
 Sunny Day Failure Inundation Map
 Sheet 7 of 10

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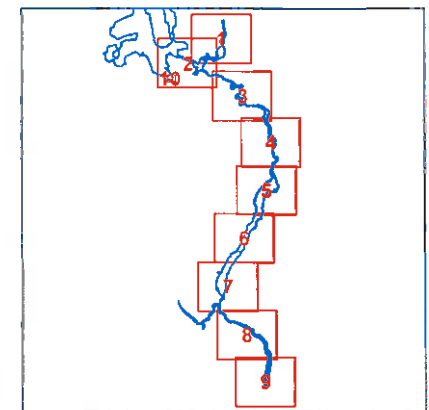


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Lake Dunstan		
	Sunny Day	Rainy Day
Time to Max. Discharge (hrs)	7.20	7.00
Time to Max. Water Level (hrs)	12.00	16.40
Max. Discharge (m ³ /s)	7,195	9,625
Max. Water Level (m RL)	197.74	199.64
Max. Water Level Rise (m)	3.74	5.64



Key Map:



Notes
 The inundated area shown is based on an hypothetical sunny day failure of the Hawea control structure.
 Time of peak discharge and water level have been rounded to the nearest 20mins.
 Maximum water levels shown assume an initial water level in Lake Wanaka of RL 277.20m for the sunny day event and RL 280.87m for the rainy day event.
 Maximum water levels for Lake Dunstan assume the Clyde Dam is operated as per Contact's Clutha flood rules.
 Flood extents at Lake Dunstan have been derived using contour data with a vertical interval of 20m, obtained from LINZ.
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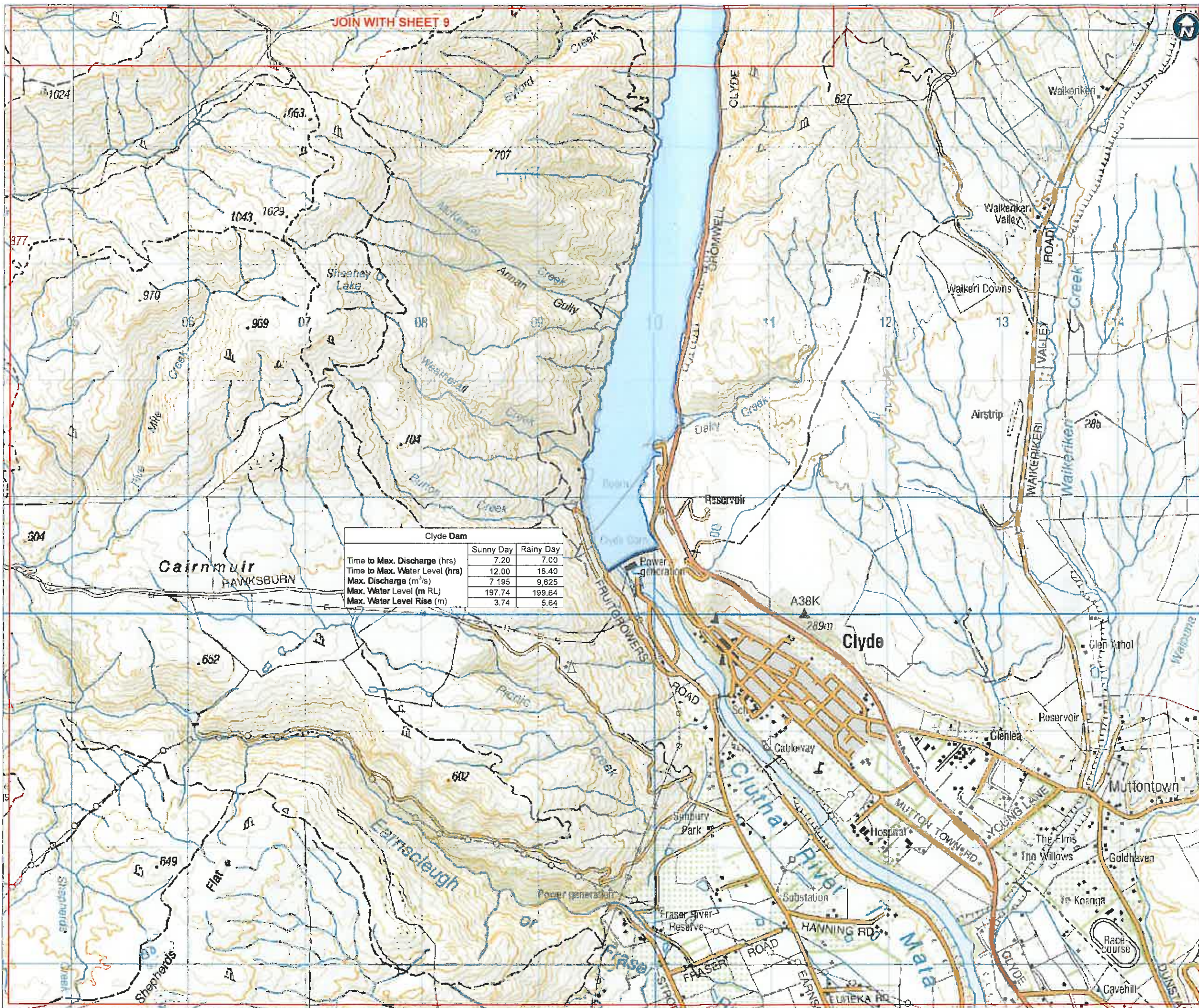
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Title
 Sunny Day Failure Inundation Map
 Sheet 8 of 10

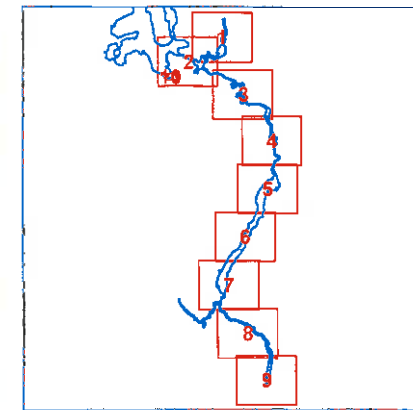
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Approved	Approved	Status		Final
File Ref.	60221308-SD-08.mxd			
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JOIN WITH SHEET 9



Key Map:



Notes

The inundated area shown is based on an hypothetical sunny day failure of the Hawea control structure. Time of peak discharge and water level have been rounded to the nearest 20mins.

Maximum water levels shown assume an initial water level in Lake Wanaka of RL 277.20m for the sunny day event and RL 280.67m for the rainy day event.

Maximum water levels for Lake Dunstan assume the Clyde Dam is operated as per Contact's Clutha flood rules.

Flood extents at Lake Dunstan have been derived using contour data with a vertical interval of 20m, obtained from LINZ. All levels are to MSL Dunedin Datum.

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Sheet No	9	Project No.	60221308	Issue	Rev
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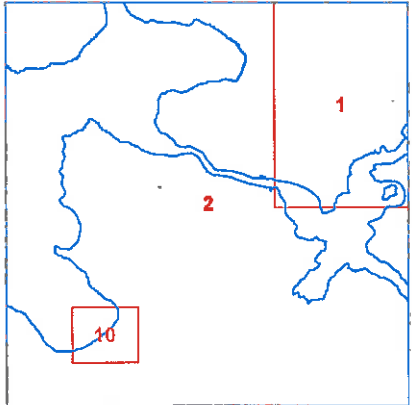
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Sunny Day Failure Inundation Map
Sheet 9 of 10

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File Ref:	60221308-SD-09.mxd			
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	Sunny Day	Rainy Day	
Initial Lake Level (m RL)	277.20	277.20	280.87
Time to Max. Inflow (hrs)	4.00	4.00	3.40
Time to Max. Water Level (hrs)	75.00	79.00	43.00
Max. Inflow (m ³ /s)	4,075	6,640	8,775
Max. Water Level (m RL)	279.22	280.20	283.45
Max. Water Level Rise (m)	2.02	3.00	2.58



Key Map:

- Legend
- Sunny Day Flood Extent
 - Rainy Day Flood Extent (Initial Lake Level 277.20m)
 - Rainy Day Flood Extent (Initial Lake Level 280.87m)
 - Map Sheets

Notes
 The inundated area shown is based on an hypothetical sunny day failure of the Hawea control structure.
 Time of peak discharge and water level have been rounded to the nearest 20mins
 Flood extents have been derived using contour data supplied by Queenstown Lakes District Council at a vertical interval of 1.0m
 All levels are to MSL, Dunedin Datum
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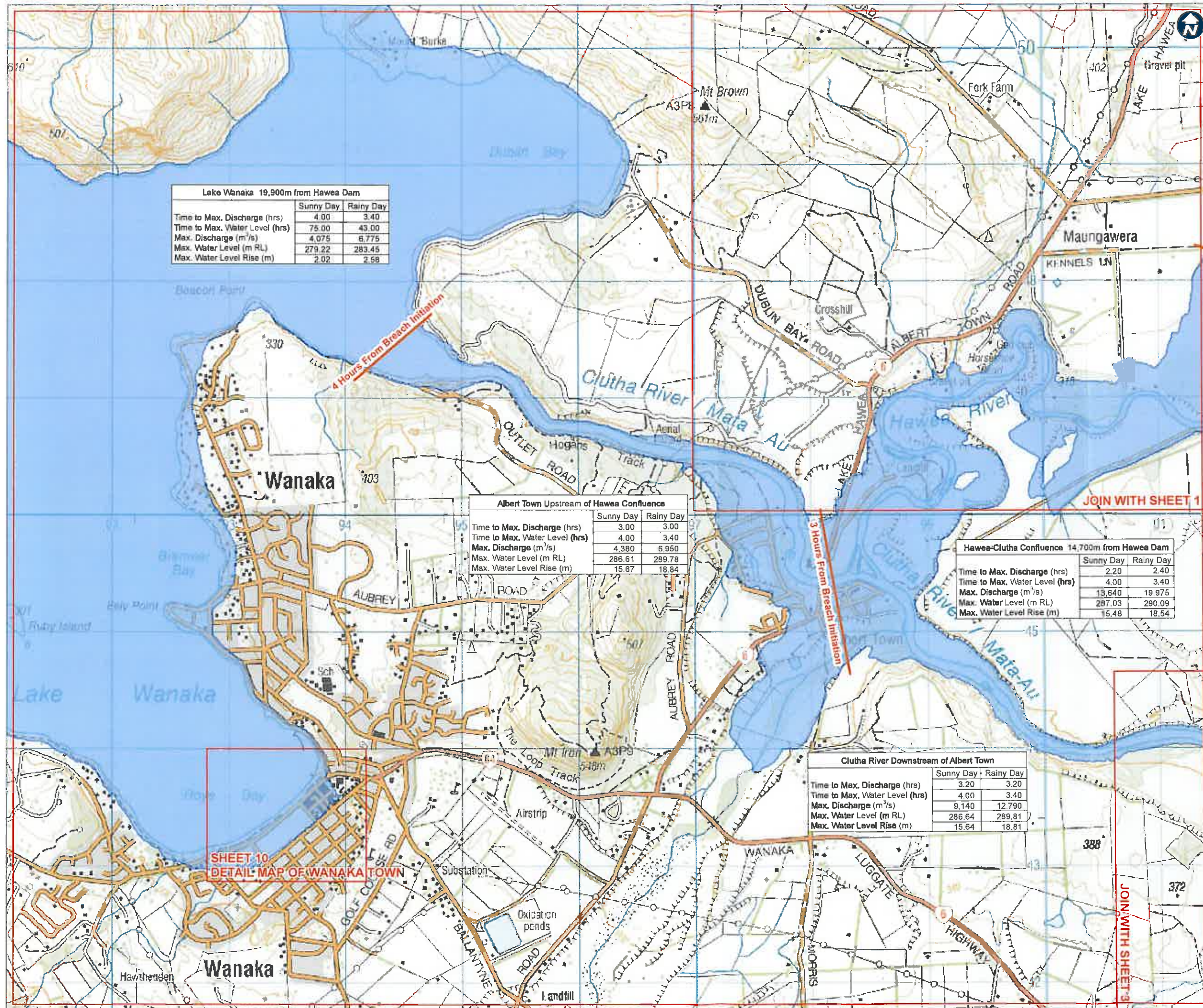
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 Project: **Lake Hawea Control Dam Dam Break Update Study**

Sheet No:	10	Project No:	60221308	Issue:	Rev
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Title: **Wanaka Town Flood Inundation Map Sunny and Rainy Day Failure Events**

Scale: 1:4,000 (A3 size)

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	PJC	PJC	Approved	GB	28 Sep 11
			Status	Final	
File Ref:	60221308-SD-10.mxd				
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Lake Wanaka 19,900m from Hawea Dam

	Sunny Day	Rainy Day
Time to Max. Discharge (hrs)	4.00	3.40
Time to Max. Water Level (hrs)	75.00	43.00
Max. Discharge (m ³ /s)	4,075	6,775
Max. Water Level (m RL)	279.22	283.45
Max. Water Level Rise (m)	2.02	2.58

Albert Town Upstream of Hawea Confluence

	Sunny Day	Rainy Day
Time to Max. Discharge (hrs)	3.00	3.00
Time to Max. Water Level (hrs)	4.00	3.40
Max. Discharge (m ³ /s)	4,380	6,950
Max. Water Level (m RL)	286.61	289.78
Max. Water Level Rise (m)	15.67	18.84

Hawea-Clutha Confluence 14,700m from Hawea Dam

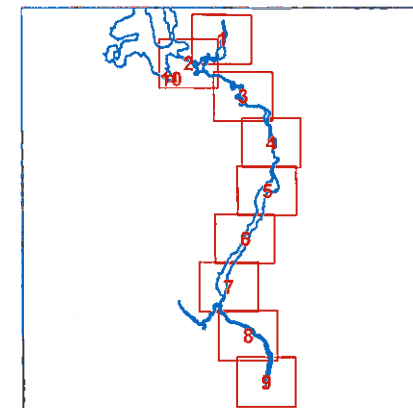
	Sunny Day	Rainy Day
Time to Max. Discharge (hrs)	2.20	2.40
Time to Max. Water Level (hrs)	4.00	3.40
Max. Discharge (m ³ /s)	13,640	19,975
Max. Water Level (m RL)	287.03	290.09
Max. Water Level Rise (m)	15.48	18.54

Clutha River Downstream of Albert Town

	Sunny Day	Rainy Day
Time to Max. Discharge (hrs)	3.20	3.20
Time to Max. Water Level (hrs)	4.00	3.40
Max. Discharge (m ³ /s)	9,140	12,790
Max. Water Level (m RL)	286.64	289.81
Max. Water Level Rise (m)	15.64	18.81

SHEET 10
DETAIL MAP OF WANAKA TOWN

JOIN WITH SHEET 9



Key Map:

Legend

- Rainy Day Flood Extent
- Flood Wave Travel Time
- Map Sheets

Notes:
 The inundated area shown is based on an hypothetical rainy day failure of the Hawea control structure.
 Time of peak discharge and water level have been rounded to the nearest 20mins.
 Maximum water levels shown assume an initial water level in Lake Wanaka of RL 277.20m for the sunny day event and RL 280.87m for the rainy day event.
 Flood extents have been derived using LIDAR data supplied by Contact Energy with a vertical accuracy of +/- 250mm.
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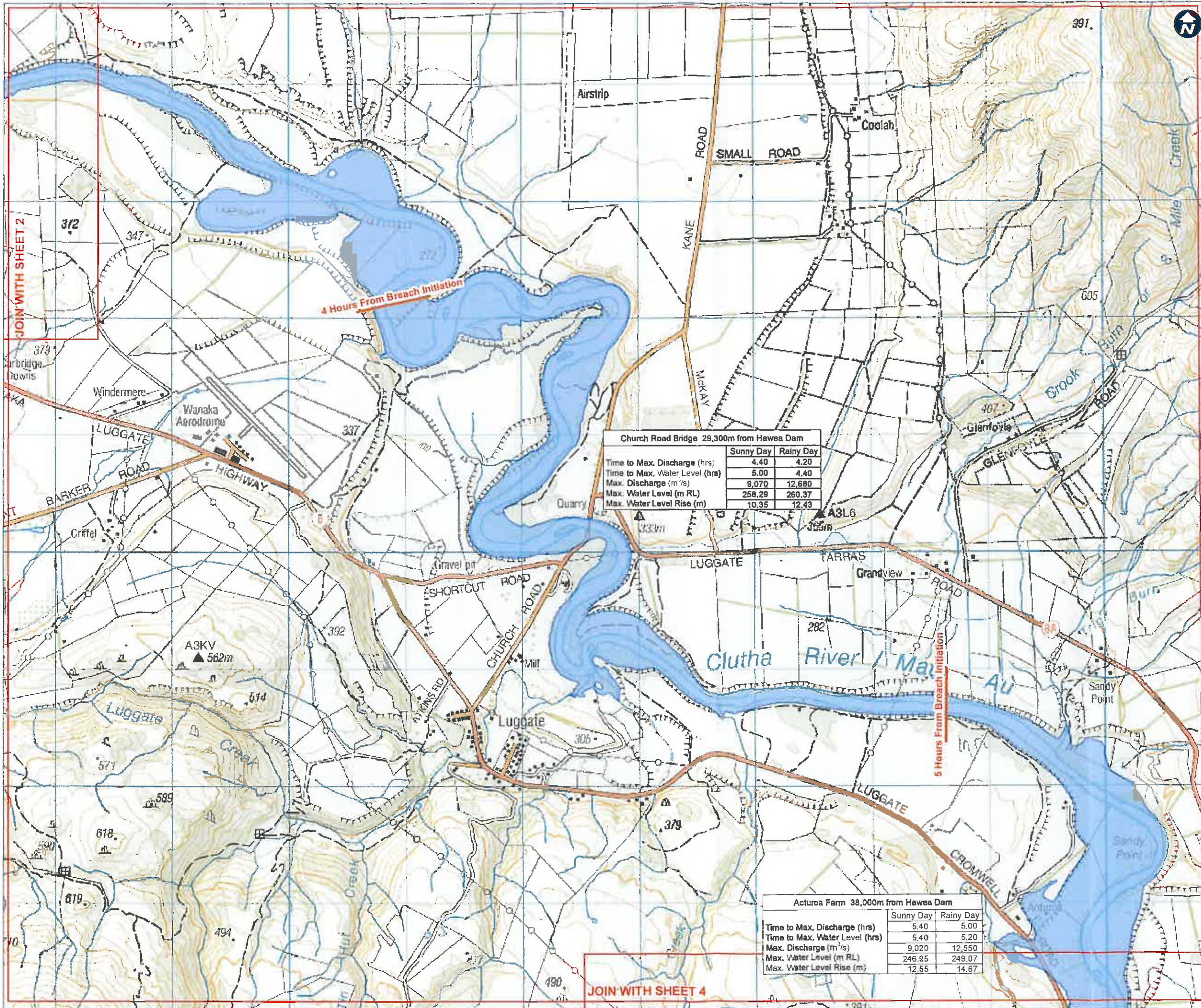
Project
**Lake Hawea Control Dam
Dam Break Update Study**

Sheet No	2	Project No	60221308	Issue	Rev
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Title
**Rainy Day Failure Inundation Map
Sheet 2 of 9**

Scale 1:30,000 (A3 size)

Design	Drawn	Approved	Checked	Date
PJC	PJC	Approved	GB	28 Sep 11
			GB	
			Status	Final
File Ref.	60221308-RD-02.mxd			
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JOIN WITH SHEET 2

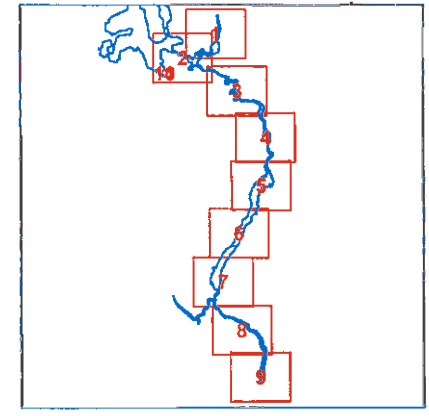
JOIN WITH SHEET 4

Church Road Bridge 29,300m from Hawea Dam

	Sunny Day	Rainy Day
Time to Max. Discharge (hrs)	4.40	4.20
Time to Max. Water Level (hrs)	5.00	4.40
Max. Discharge (m ³ /s)	9,070	12,680
Max. Water Level (m RL)	258.29	260.37
Max. Water Level Rise (m)	10.35	12.43

Acturoa Farm 38,000m from Hawea Dam

	Sunny Day	Rainy Day
Time to Max. Discharge (hrs)	5.40	5.00
Time to Max. Water Level (hrs)	5.40	5.20
Max. Discharge (m ³ /s)	9,020	12,550
Max. Water Level (m RL)	246.95	249.07
Max. Water Level Rise (m)	12.55	14.67



Key Map:

- Legend
- Rainy Day Failure Extent
 - Flood Wave Travel Time
 - Map Sheets

Notes
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 Time of peak discharge and water level have been rounded to the nearest 20mins.
 Maximum water levels shown assume an initial water level in Lake Wanaka of RL 277.20m for the sunny day event and RL 280.87m for the rainy day event.
 Flood extents have been derived using LIDAR data supplied by Contact Energy with a vertical accuracy of +/- 250mm.
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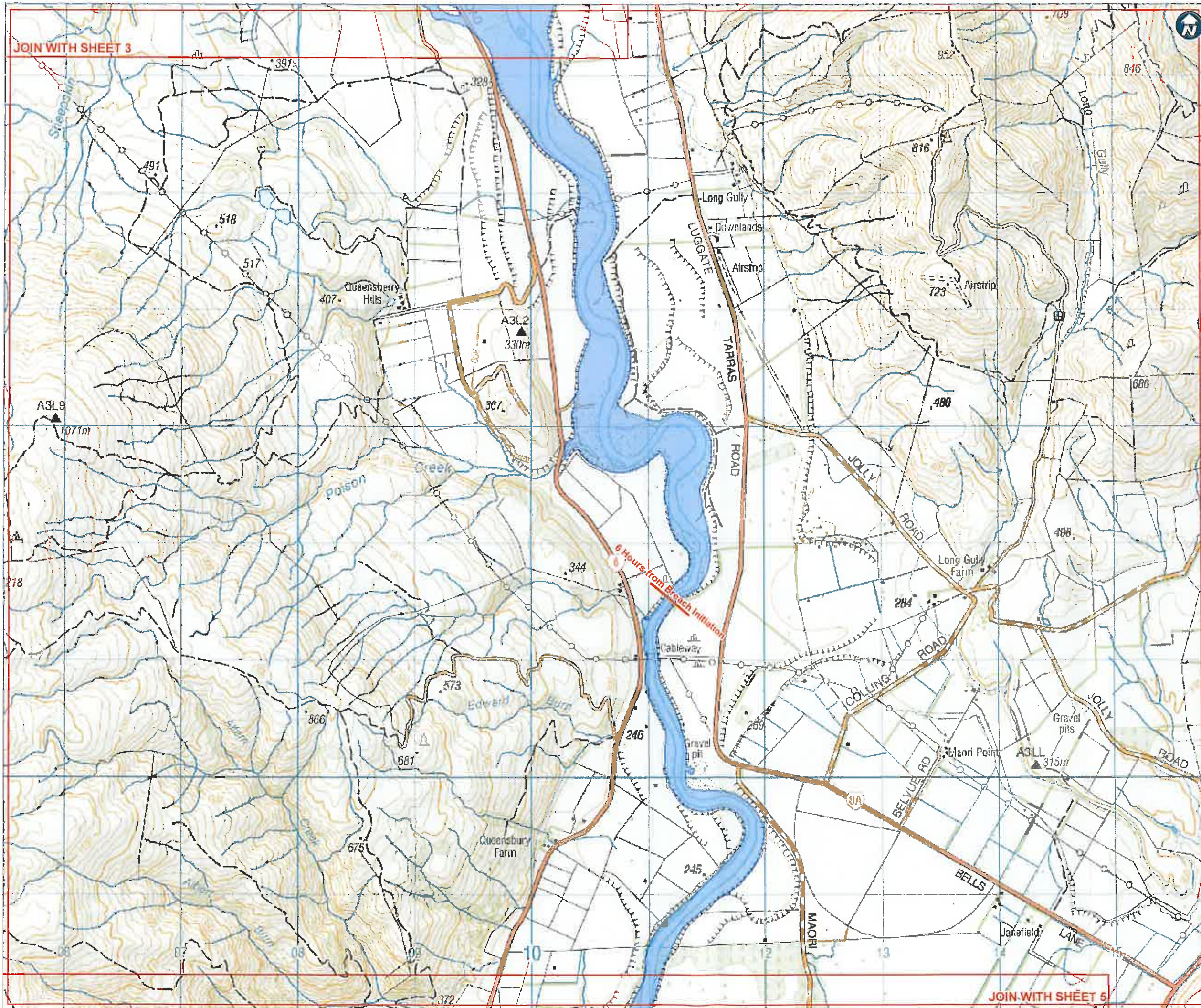
Sheet No: 3 | Project No: 60221308 | Issue Rev

Title: **Rainy Day Failure Inundation Map Sheet 3 of 9**

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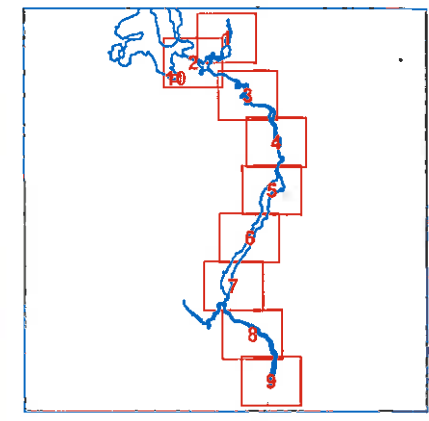
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Drawn	PJC	GB	GB	28 Sep 11
Approved	Approved	Status	Final	
File Ref.	60221308-RD-03.mxd			
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JOIN WITH SHEET 3

JOIN WITH SHEET 5



Key Map:

- Legend
- Rainy Day Flood Extent
 - Flood Wave Travel Time
 - Map Sheets

Notes
 The inundated area shown is based on an hypothetical rainy day failure of the Hawea control structure.
 Time of peak discharge and water level have been rounded to the nearest 20mins
 Maximum water levels shown assume an initial water level in Lake Waikata of RL 277.20m for the sunny day event and RL 280.87m for the rainy day event
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 All levels are to MSL, Dunedin Datum
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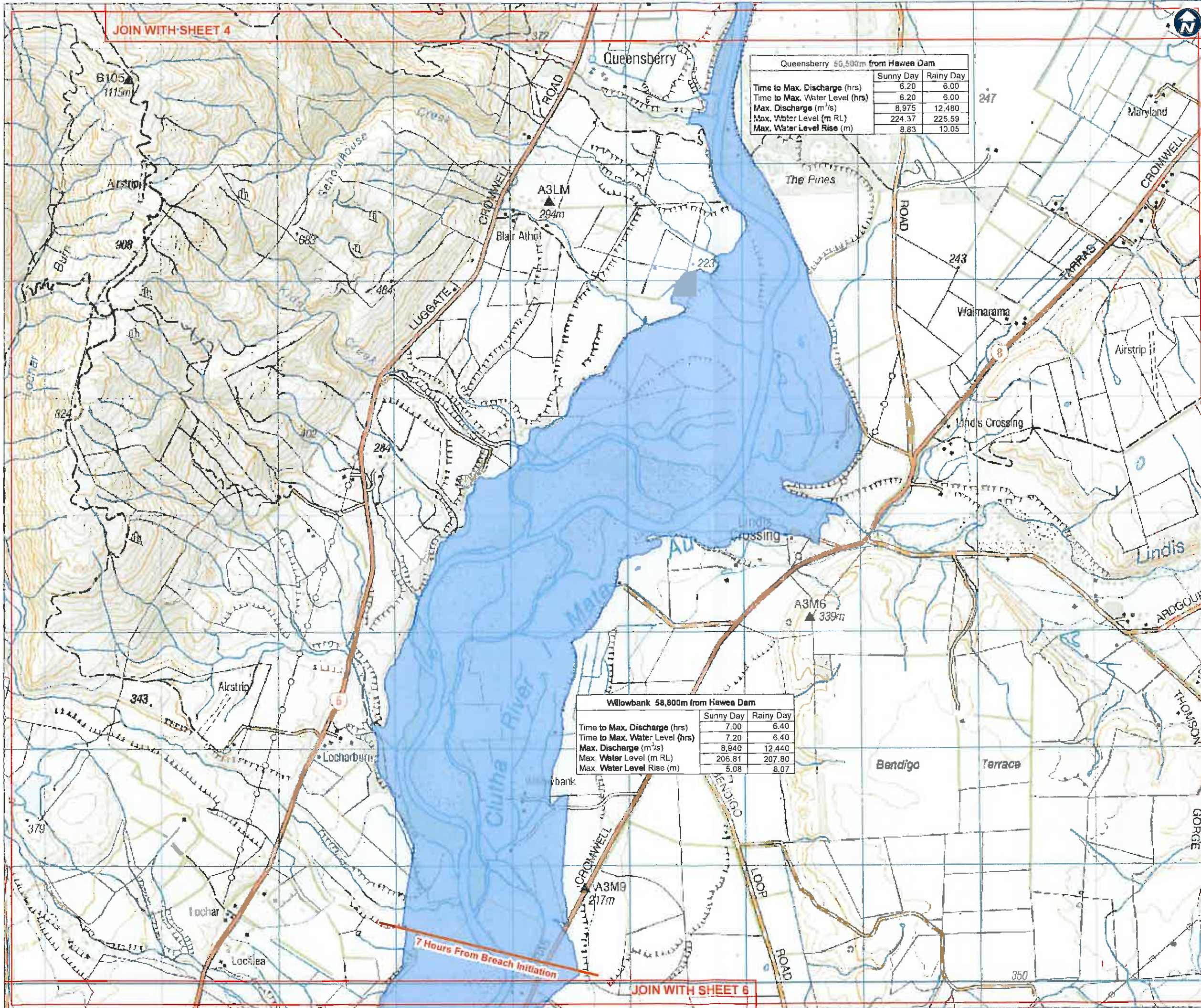
Client
Contact Energy
 Project
**Lake Hawea Control Dam
 Dam Break Update Study**

Sheet No	4	Project No.	60221308	Issue	Rev
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Title
**Rainy Day Failure Inundation Map
 Sheet 4 of 9**

Scale 1:30,000 (A3 size)

Design	PJC	Checked	GB	Date
Drawn	PJC		GB	28 Sep 11
Approved	Approved	Status	Final	
File Ref.	60221308-RD-04.mxd			
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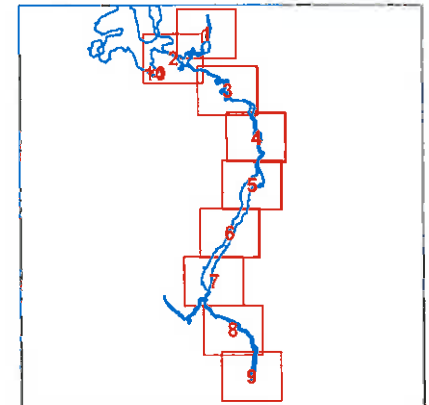
JOIN WITH SHEET 4

Queensberry 50,500m from Hawea Dam		
	Sunny Day	Rainy Day
Time to Max. Discharge (hrs)	6.20	6.00
Time to Max. Water Level (hrs)	6.20	6.00
Max. Discharge (m ³ /s)	8,975	12,480
Max. Water Level (m RL)	224.37	225.59
Max. Water Level Rise (m)	8.83	10.05

Willowbank 58,800m from Hawea Dam		
	Sunny Day	Rainy Day
Time to Max. Discharge (hrs)	7.00	6.40
Time to Max. Water Level (hrs)	7.20	6.40
Max. Discharge (m ³ /s)	8,940	12,440
Max. Water Level (m RL)	206.81	207.80
Max. Water Level Rise (m)	5.08	6.07

7 Hours From Breach Initiation

JOIN WITH SHEET 6



Key Map:

- Legend
- Rainy Day Flood Extent
 - Flood Wave Travel Time
 - Map Sheets

Notes
 The inundated area shown is based on an hypothetical rainy day failure of the Hawea control structure.
 Time of peak discharge and water level have been rounded to the nearest 20mins
 Maximum water levels shown assume an initial water level in Lake Wanaka of RL 277.20m for the sunny day event and RL 280.87m for the rainy day event
 Flood extents have been derived using LIDAR data supplied by Contact Energy with a vertical accuracy of +/- 250mm
 All levels are to MSL Dunedin Datum
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 Project: **Lake Hawea Control Dam Dam Break Update Study**

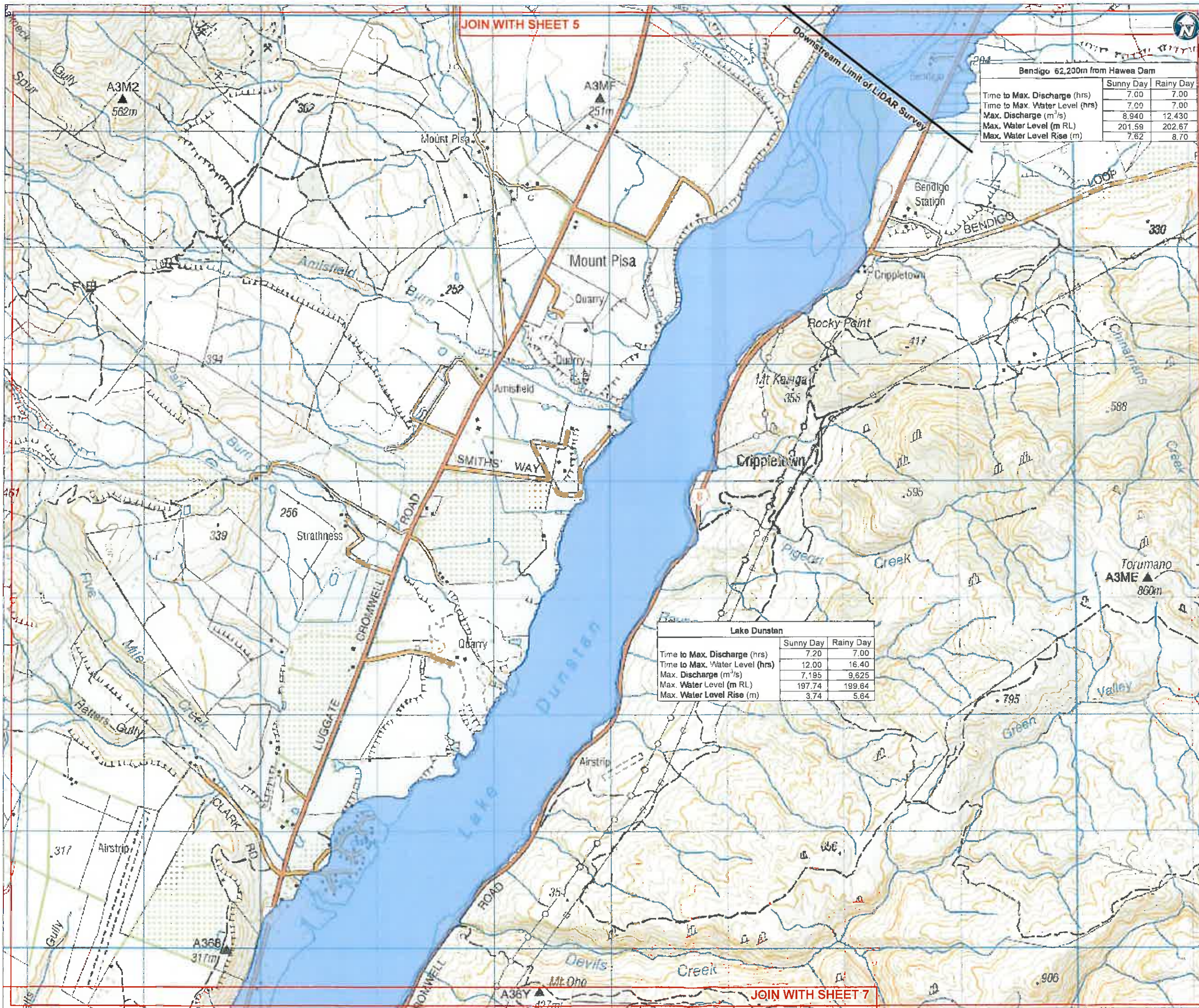
Sheet No. 5	Project No. 60221308	Issue Rev
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Title: **Rainy Day Failure Inundation Map Sheet 5 of 9**

Scale: 1:30,000 (A3 size)

Design	Checked	Date
PJC	GB	28 Sep 11
Drawn	GB	
Approved	Status	Final
File Ref	60221308-RD-05.mxd	
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JOIN WITH SHEET 5

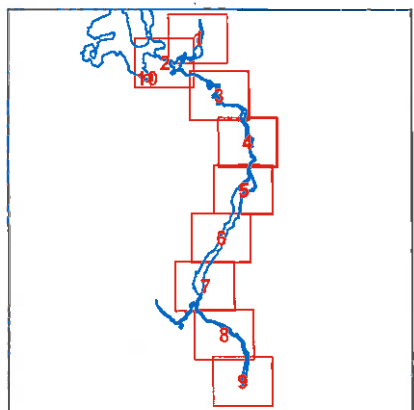


Bendigo 62,200m from Hawea Dam

	Sunny Day	Rainy Day
Time to Max. Discharge (hrs)	7.00	7.00
Time to Max. Water Level (hrs)	7.00	7.00
Max. Discharge (m ³ /s)	8,940	12,430
Max. Water Level (m RL)	201.59	202.67
Max. Water Level Rise (m)	7.62	8.70

Lake Dunstan

	Sunny Day	Rainy Day
Time to Max. Discharge (hrs)	7.20	7.00
Time to Max. Water Level (hrs)	12.00	16.40
Max. Discharge (m ³ /s)	7,195	9,625
Max. Water Level (m RL)	197.74	199.64
Max. Water Level Rise (m)	3.74	5.64



Key Map:

- Legend
- Rainy Day Flood Extent
 - Flood Wave Travel Time
 - Map Sheets

Notes

The inundated area shown is based on an hypothetical rainy day failure of the Hawea control structure.

Time of peak discharge and water level have been rounded to the nearest 20mins.

Maximum water levels shown assume an initial water level in Lake Wanaka of RL 277.20m for the sunny day event and RL 280.87m for the rainy day event.

Maximum water levels for Lake Dunstan assume the Clyde Dam is operated as per Contact's Clutha flood rules.

Flood extents at Lake Dunstan have been derived using contour data with a vertical interval of 20m, obtained from LINZ.

All levels are to MSL, Dunedin Datum.

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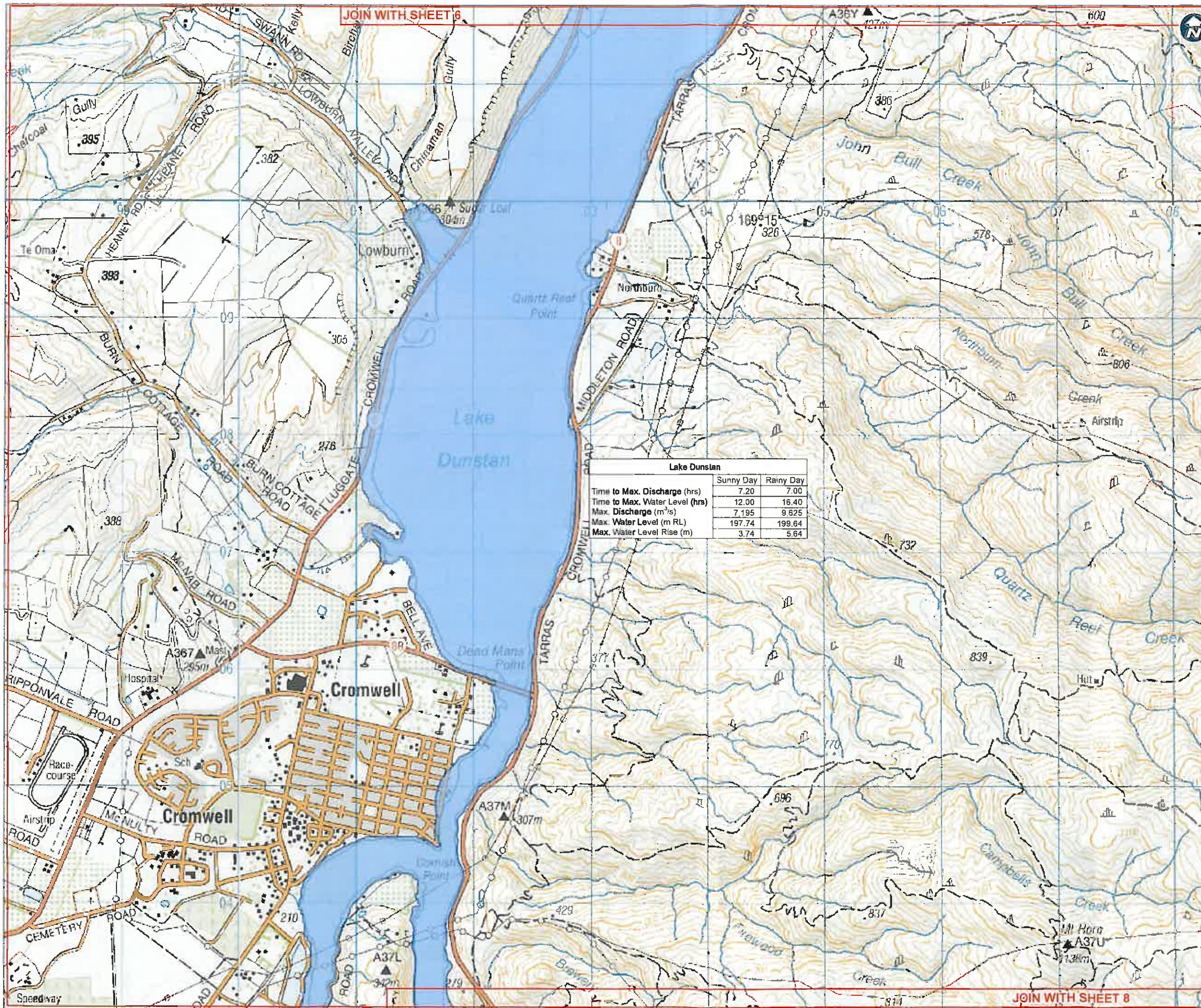
Sheet No	8	Project No	60221308	Issue	Rev
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Title
**Rainy Day Failure Inundation Map
Sheet 6 of 9**

Scale 1:30,000 (A3 size)

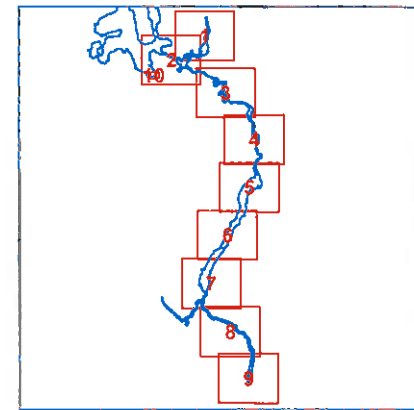
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Drawn	PJC	GB
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JOIN WITH SHEET 7

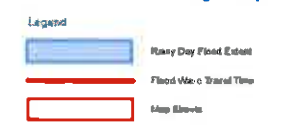


JOIN WITH SHEET 6

JOIN WITH SHEET 8



Key Map:



Notes
 The inundated area shown is based on an hypothetical rainy day failure of the Hawea control structure. Times of peak discharge and water level have been rounded to the nearest 20mins.
 Maximum water levels shown assume an initial water level in Lake Wanaka of RL 277.20m for the sunny day event and RL 280.87m for the rainy day event.
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 Flood extents at Lake Dunstan have been derived using contour data with a vertical interval of 20m, obtained from LINZ.
 All levels are to MSL Dunedin Datum.
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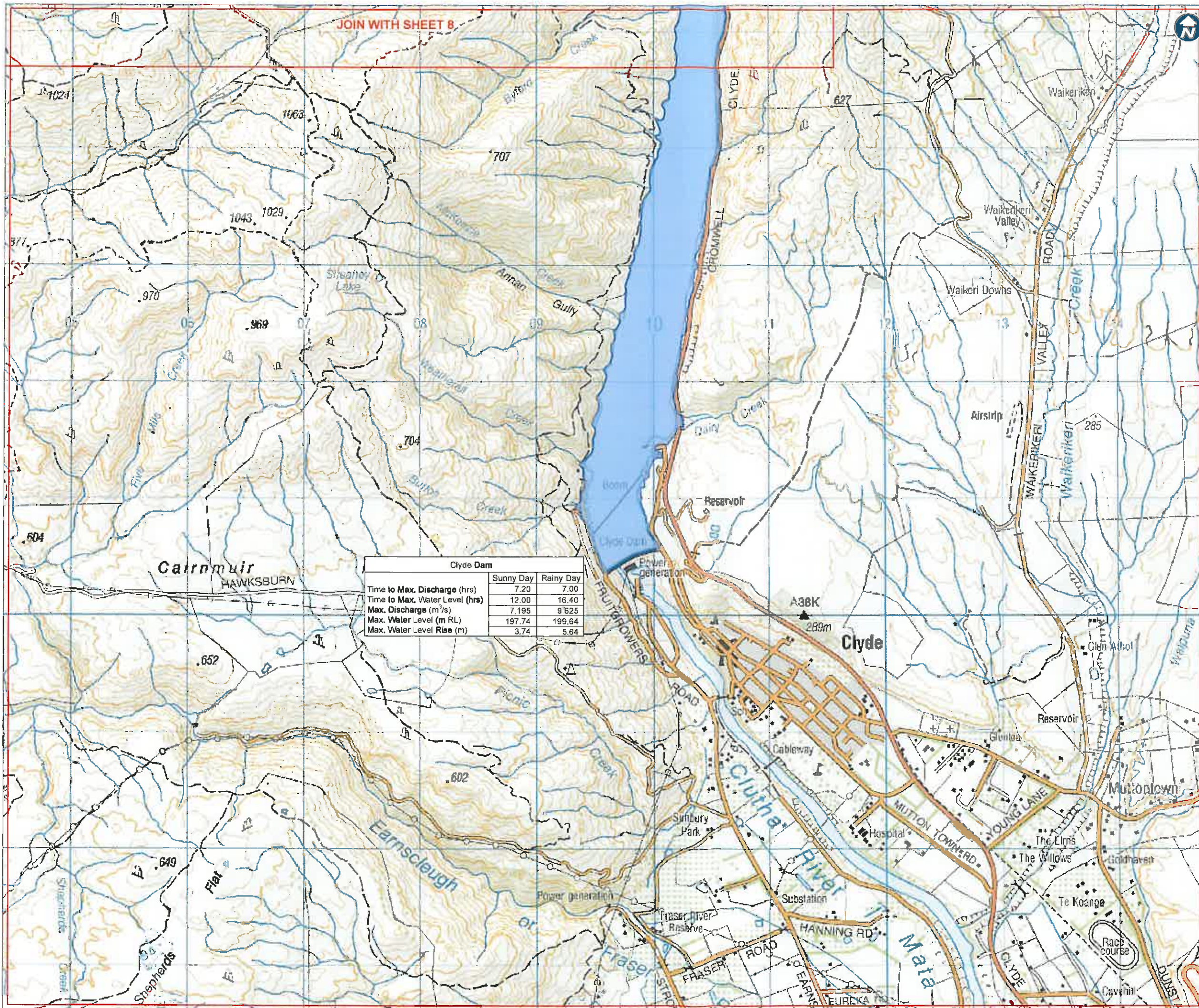
Client: **Contact Energy**
 Project: **Lake Hawea Control Dam Dam Break Update Study**

Sheet No: 7 Project No: 60221308 Issue Rev

Title: **Rainy Day Failure Inundation Map Sheet 7 of 9**

Scale 1:30,000 (A3 size)

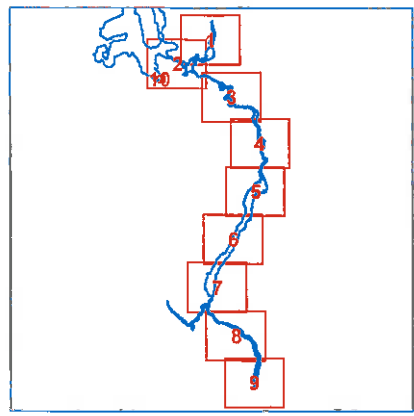
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Drawn	PJC		GB	28 Sep 11
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File Ref.	60221308-RD-07.mxd			
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JOIN WITH SHEET 8



Clyde Dam		
	Sunny Day	Rainy Day
Time to Max. Discharge (hrs)	7.20	7.00
Time to Max. Water Level (hrs)	12.00	16.40
Max. Discharge (m ³ /s)	7,195	9,625
Max. Water Level (m RL)	197.74	199.64
Max. Water Level Rise (m)	3.74	5.64



Key Map:

Legend

- Rainy Day Flood Extent
- Flood Wave Travel Time
- Map Sheet

Notes
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Sheet No	9	Project No	60221308	Issue	Rev
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Title
**Rainy Day Failure Inundation Map
 Sheet 9 of 9**

Scale: 1:30,000 (A3 size)

	Checked	Date
Design	PJC	GB 28 Sep 11
Drawn	PJC	GB
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